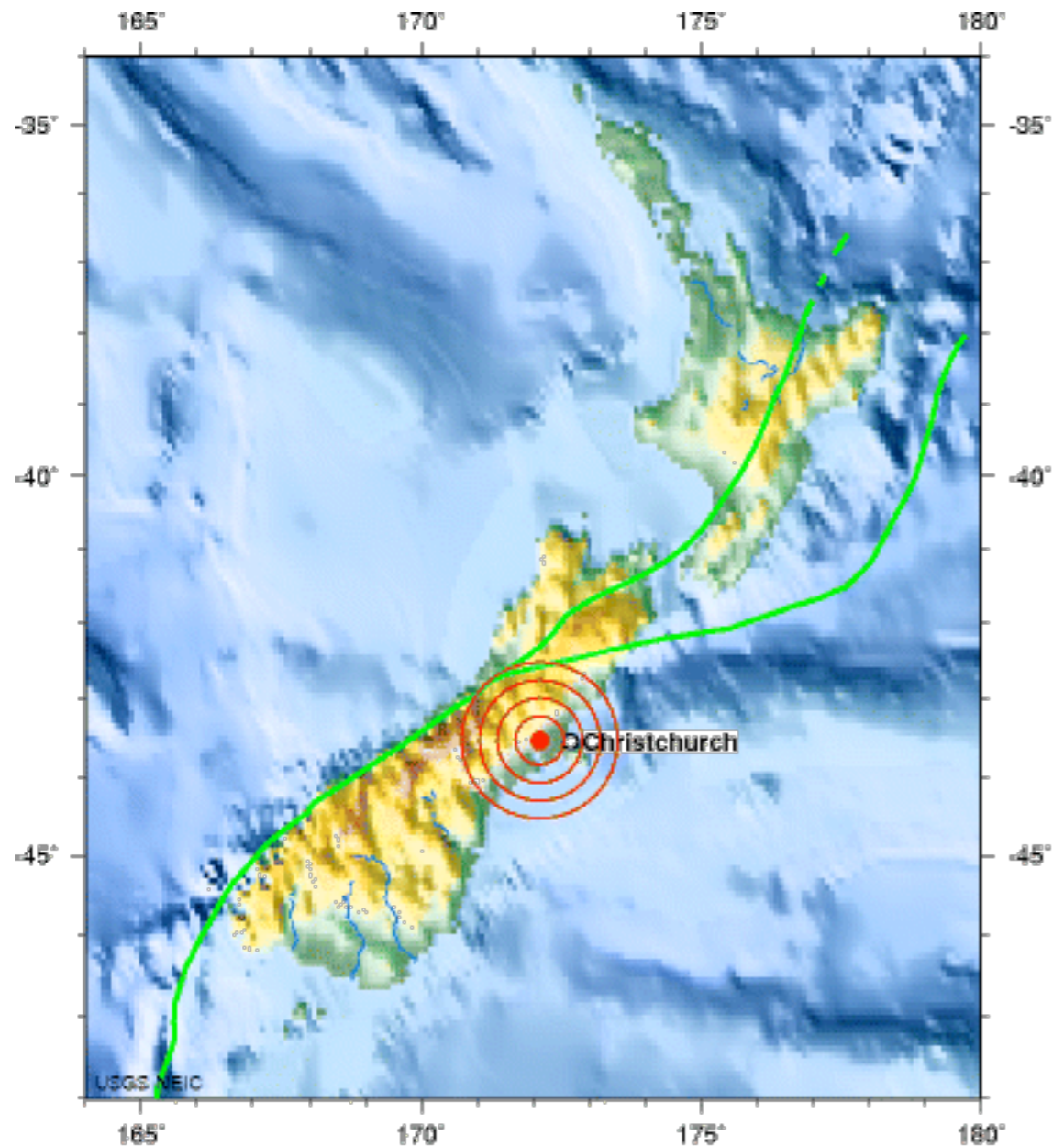


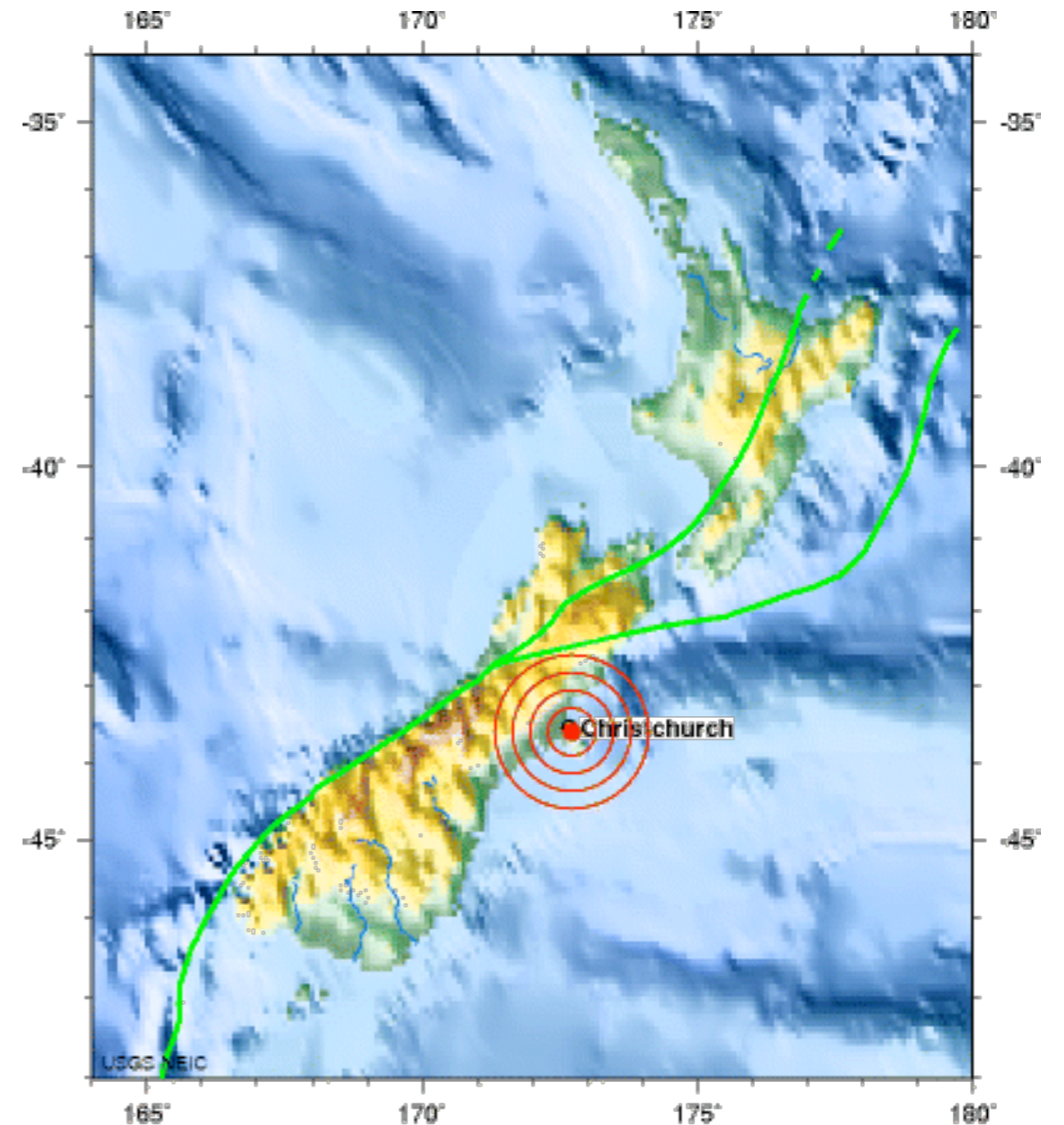
Mainshock: M 7

Aftershock: M 6.3



SOUTH ISLAND OF NEW ZEALAND
2010 09 03 16:35:46 UTC 43.53S 172.12E Depth: 5.0 km, Magnitude: 7.0
Earthquake Location

Darfield NZ Sept 3, 2010



SOUTH ISLAND OF NEW ZEALAND
2011 02 21 23:51:43 UTC 43.60S 172.71E Depth: 5.0 km
Earthquake Location

Christchurch NZ Feb 22, 2011



UC
UNIVERSITY OF
CANTERBURY
Te Whare Wānanga o Waitaha
CHRISTCHURCH NEW ZEALAND

GNS
SCIENCE
14 JUL 2010

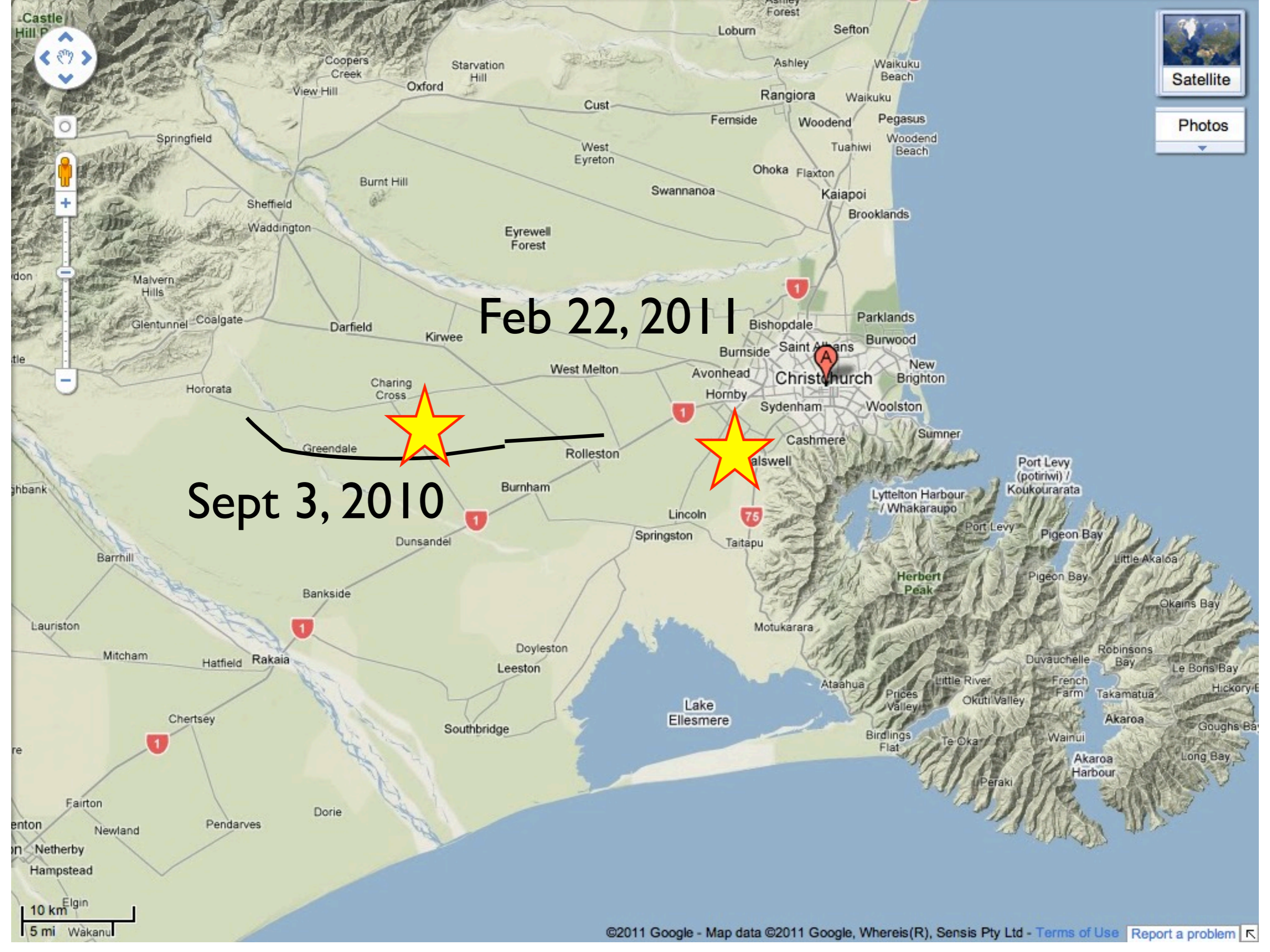
Christchurch

Rolleston

NO FAULT
HERE yechire

0 5 km

M 7 Darfield Quake from last September



Feb 22, 2011

Sept 3, 2010

Darfield NZ Sept 3, 2010 Mainshock: M 7



Aftershock: M 6.3 Christchurch NZ Feb 22, 2011



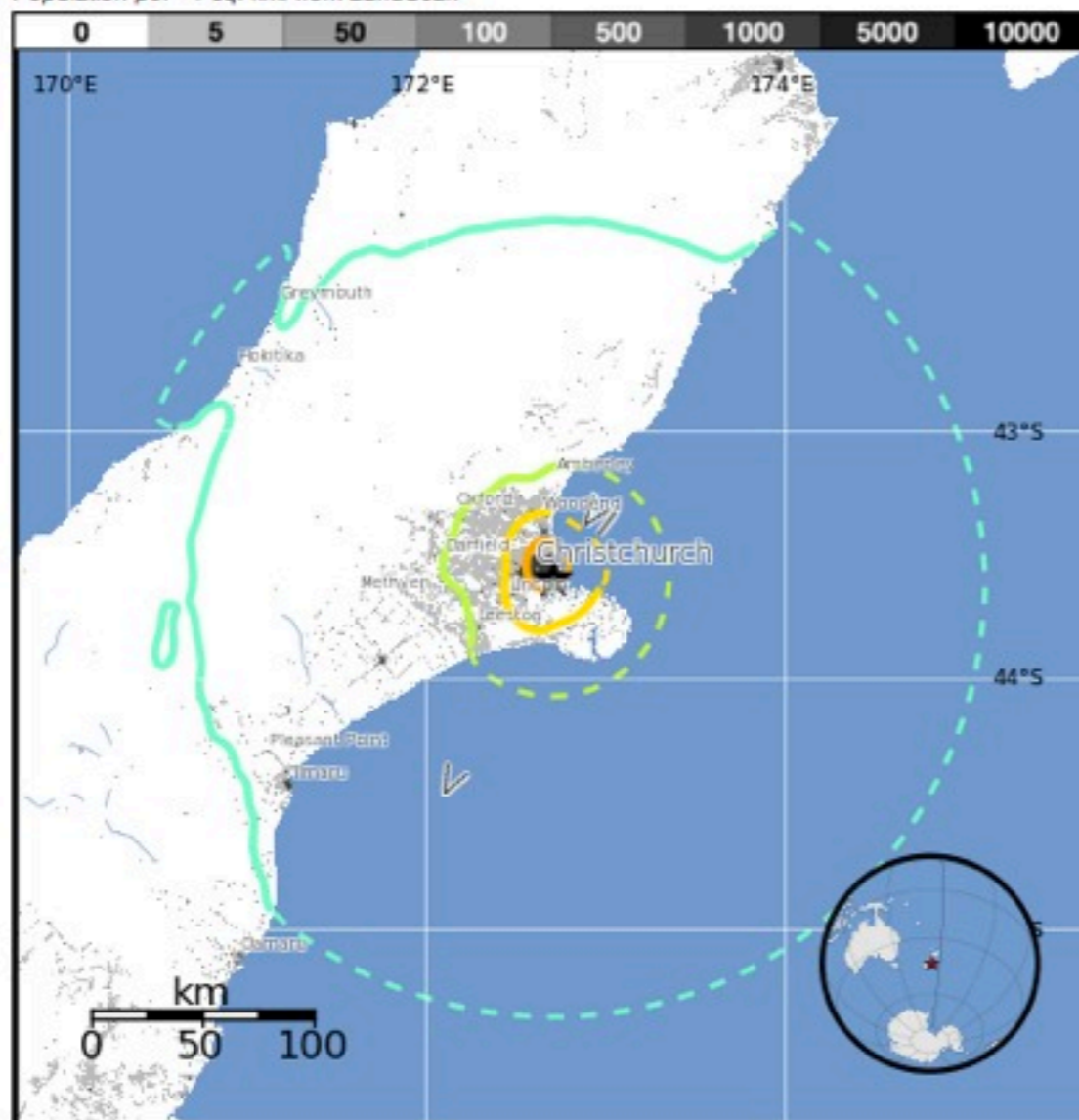
Estimated Population Exposed to Earthquake Shaking

Estimated Modified Mercalli Intensity		I	II-III	IV	V	VI	VII	VIII	IX	X
Est. Population Exposure		---	---	161k*	94k	54k	68k	245k	65k	0
Perceived Shaking		Not Felt	Weak	Light	Moderate	Strong	Very Strong	Severe	Violent	Extreme
Potential Structure Damage	Resistant	none	none	none	V. Light	Light	Moderate	Moderate/Heavy	Heavy	V. Heavy
	Vulnerable	none	none	none	Light	Moderate	Moderate/Heavy	Heavy	V. Heavy	V. Heavy

*Estimated exposure only includes population within calculated shake map area

Population Exposure

Population per ~1 sq. km. from LandScan



Structure Information Summary

Overall, the population in this region resides in structures that are highly resistant to earthquake shaking, though some vulnerable structures exist. The predominant vulnerable building types are reinforced masonry and concrete/cinder block masonry construction.

Historical Earthquakes (with MMI)

Date (UTC)	Dist (km)	Mag	Max MMI (#)	Shaking Deaths
1984-06-24	164	6.1	VIII (18)	0
1990-02-10	143	6	VIII (61)	0
1994-06-19	100	5.9	VIII (12)	0

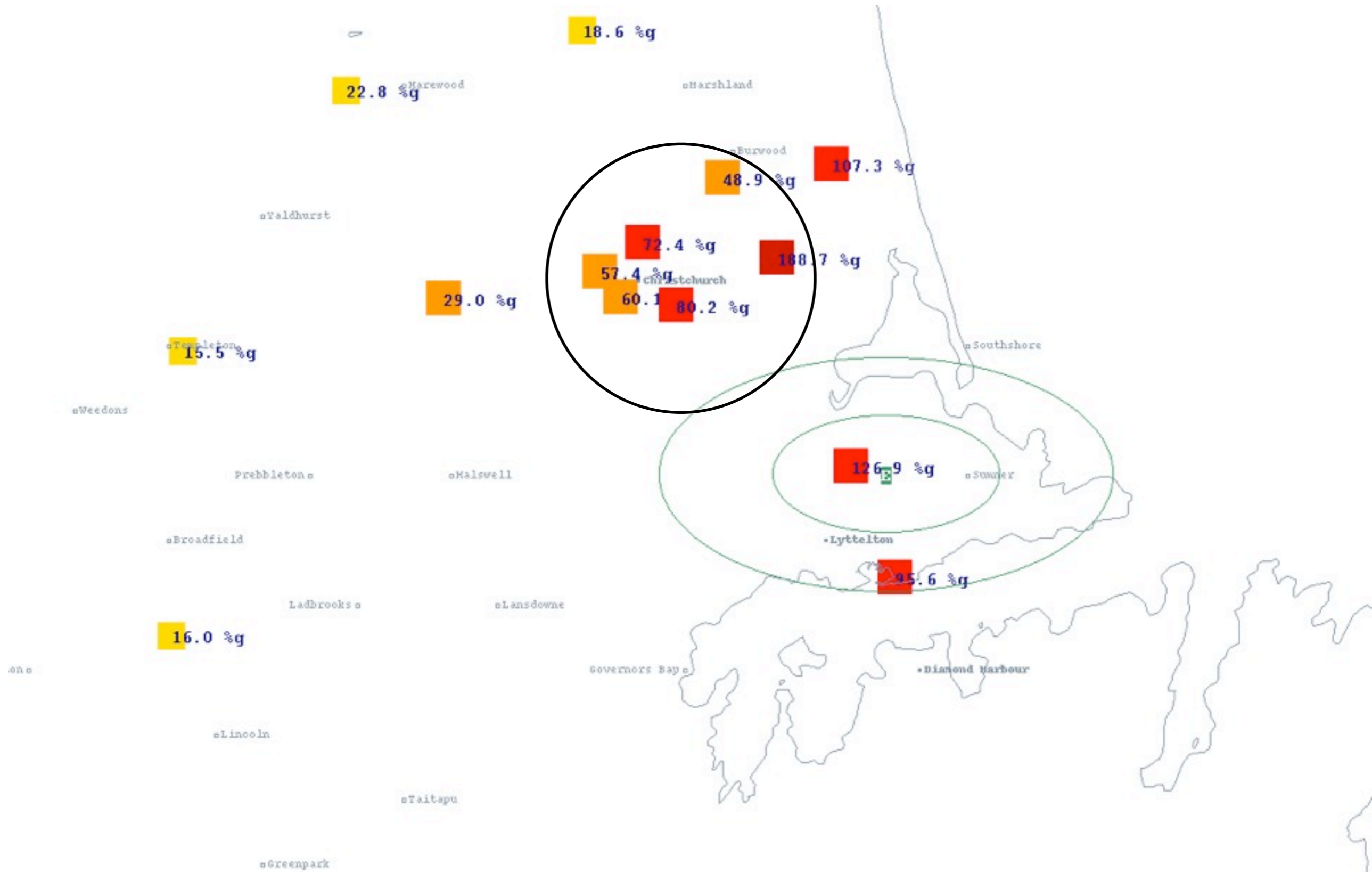
Selected Cities Exposed

from GeoNames Database of Cities with 1,000 or more residents.

MMI	City	Population
VIII	Christchurch	364k
VI	Woodend	3k
VI	Lincoln	2k
V	Rolleston	3k
V	Oxford	2k
V	Darfield	2k
IV	Timaru	28k
IV	Blenheim	27k

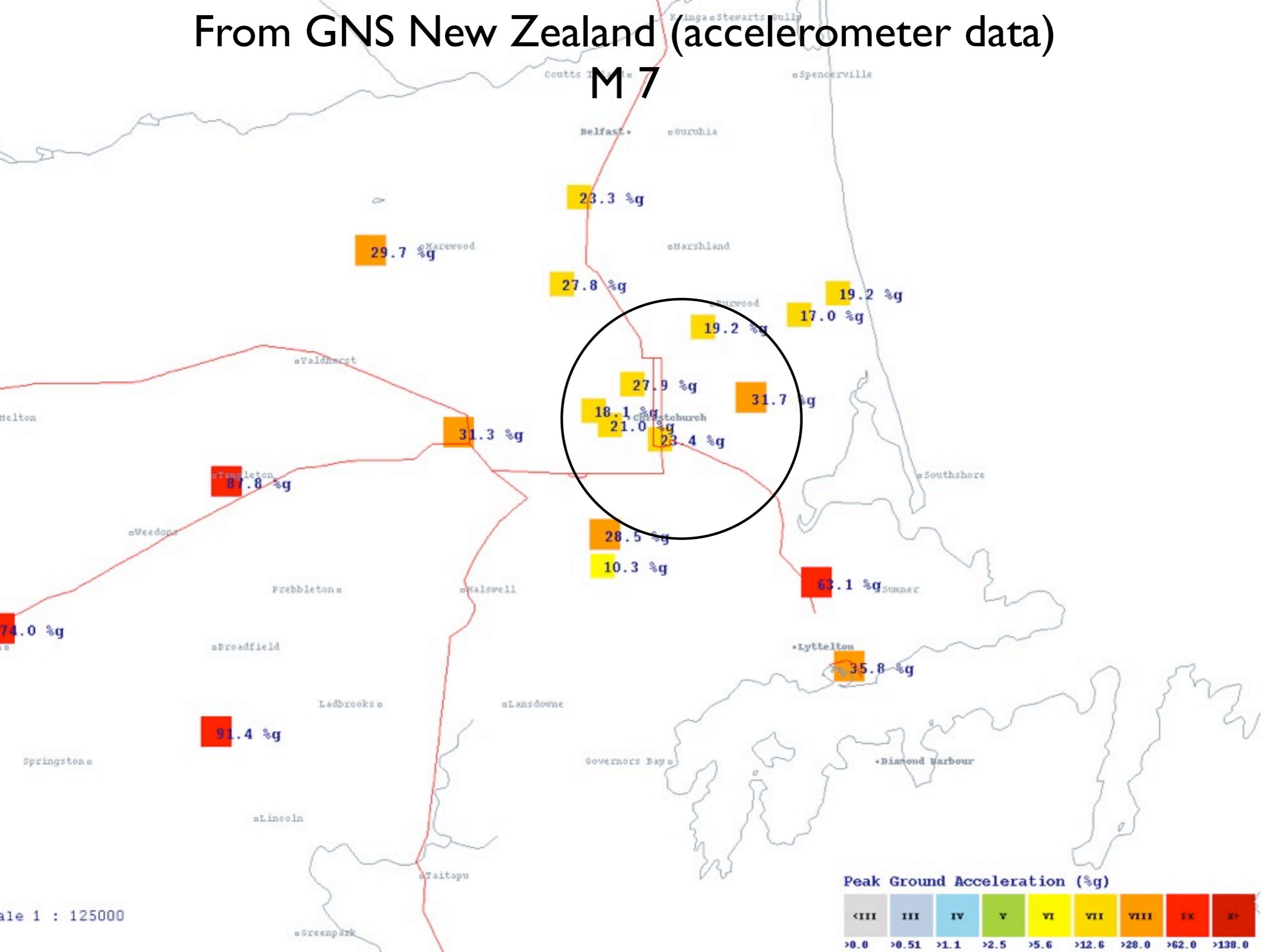
[http://www.youtube.com/watch?
v=Wrmj5UVMylI&feature=youtu.be](http://www.youtube.com/watch?v=Wrmj5UVMylI&feature=youtu.be)

From GNS New Zealand (accelerometer data) M 6.3

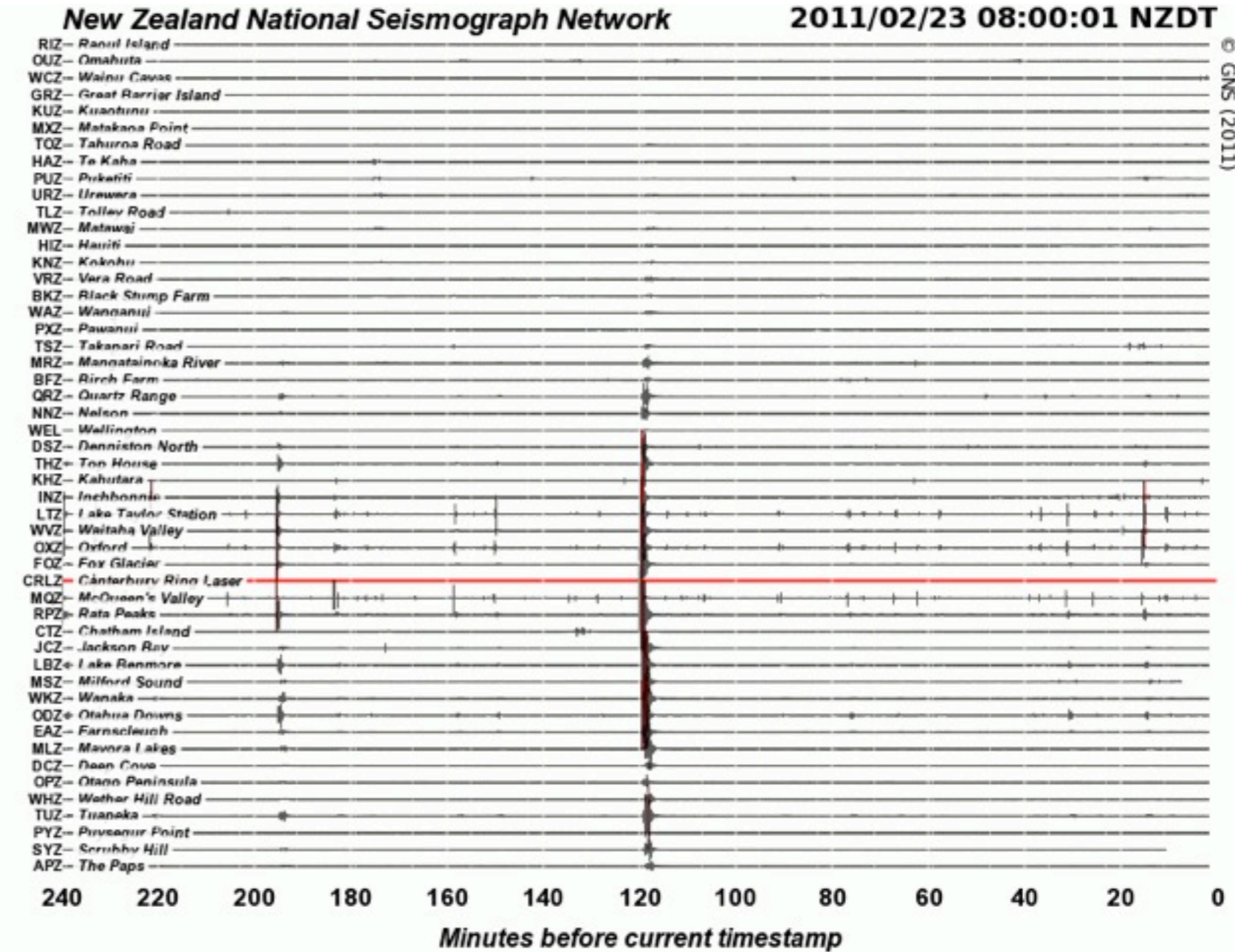


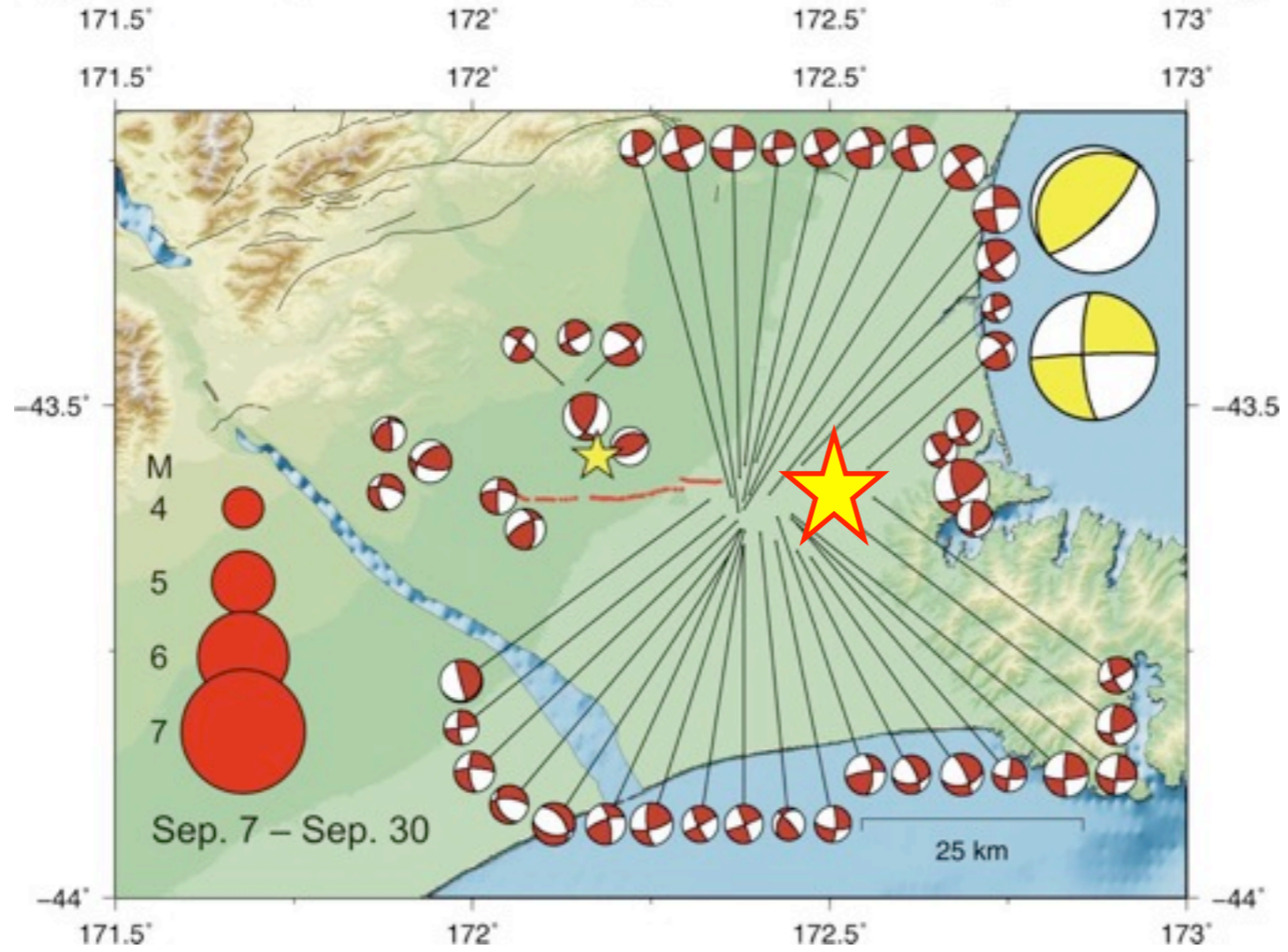
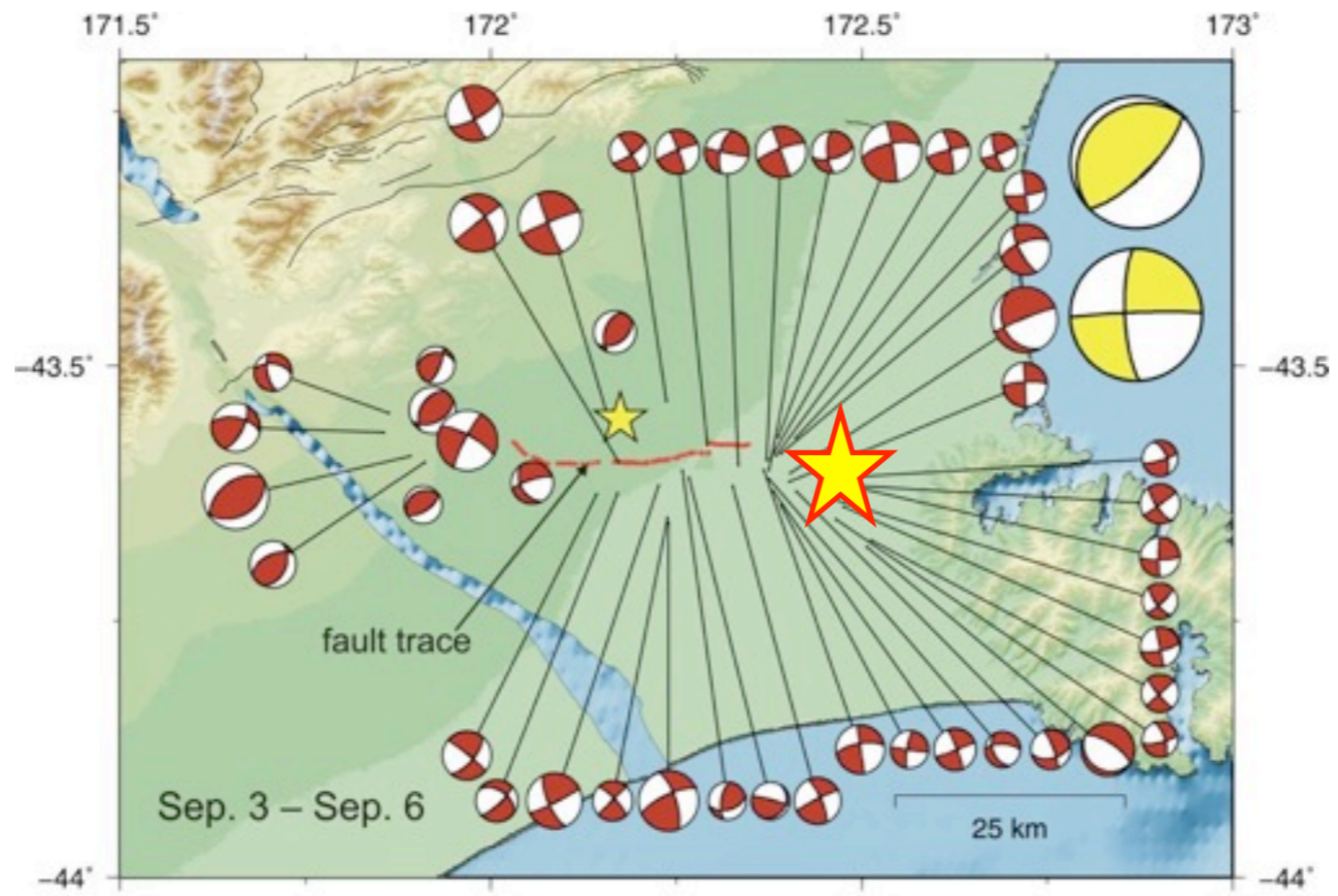
From GNS New Zealand (accelerometer data)

M7

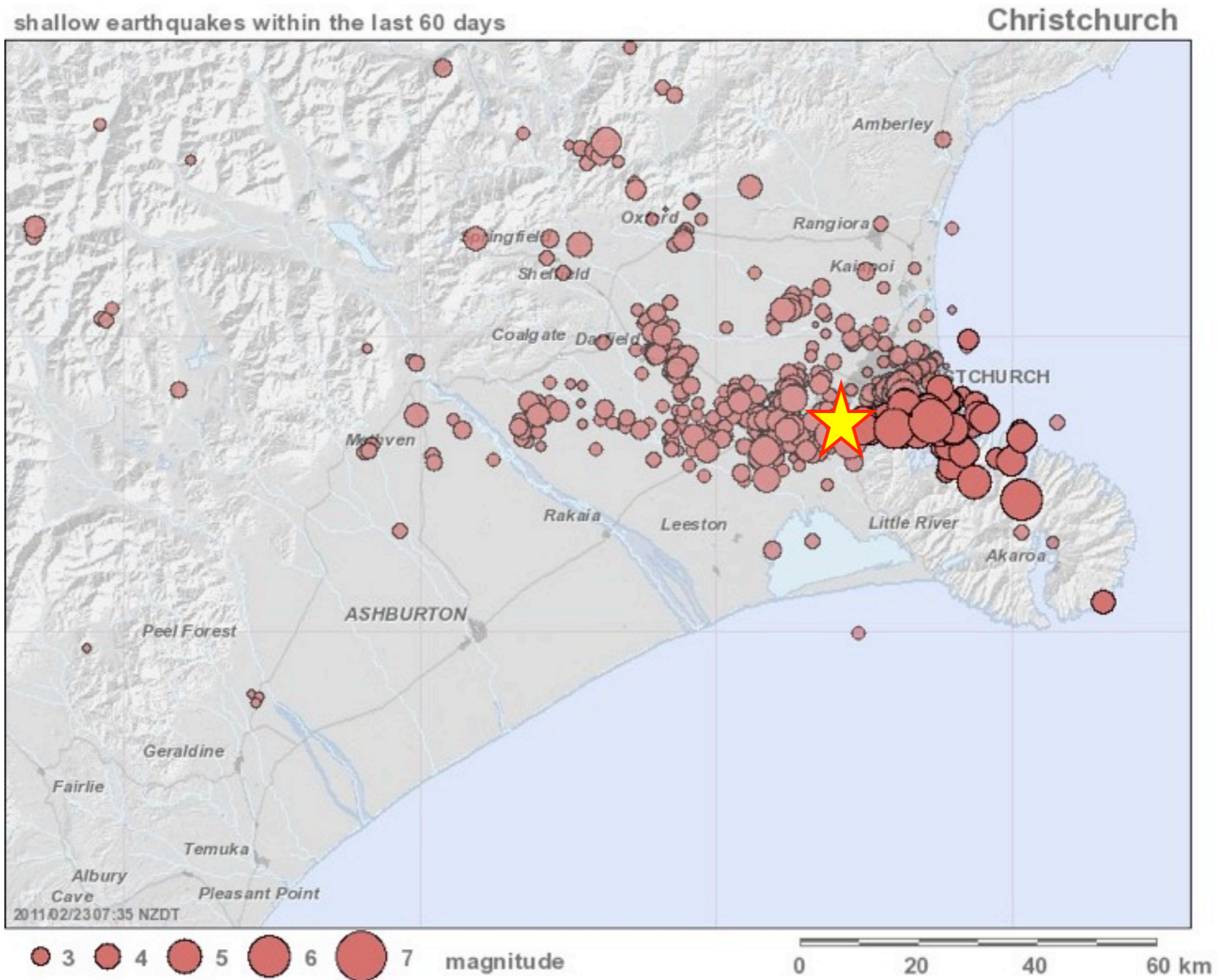


New Zealand Seismograph network





From GNS New Zealand - later part of Darfield aftershock sequence



Strong motion I

Strong motion parameters - peak values, Fourier spectra,
corner frequency
estimating slipped patch size and stress drop

Response spectra - what are they and what are they for

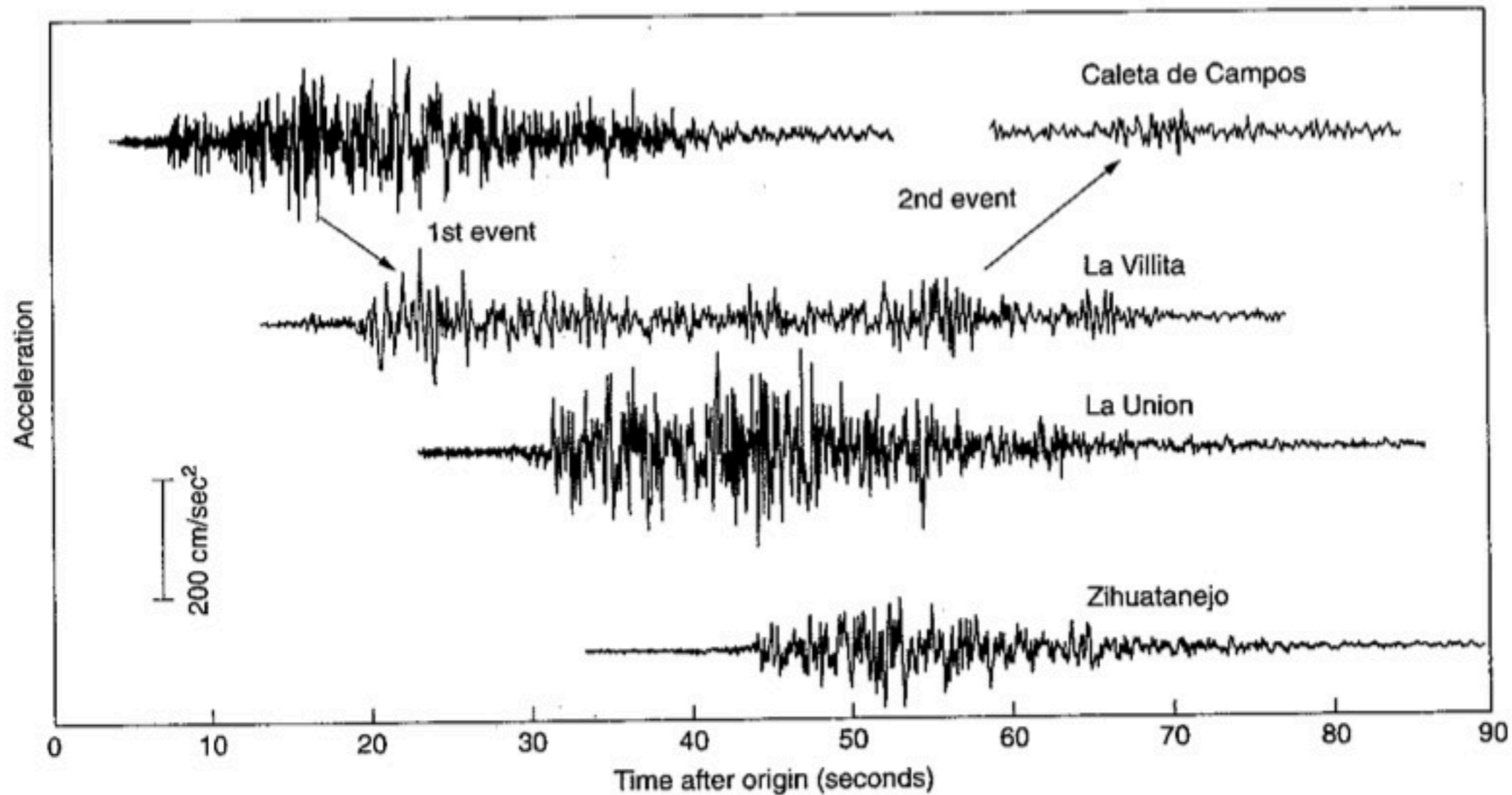


FIGURE 1 North-south component of acceleration at four strong-motion stations that recorded the September 19, 1985, Michoacan, Mexico, earthquake ($M_w = 8.0$). The Caleta de Campos station is almost directly above the hypocenter, the Zihuatanejo station is near the end of the rupture, 146 km east-southeast of Caleta de Campos, and the other stations are approximately along a line between Caleta de Campos and Zihuatanejo. The vertical separation of the traces is proportional to the separation of the stations on a projection along the strike of the causative fault. All records are aligned in absolute time after the origin of the earthquake (from Anderson *et al.*, 1986).

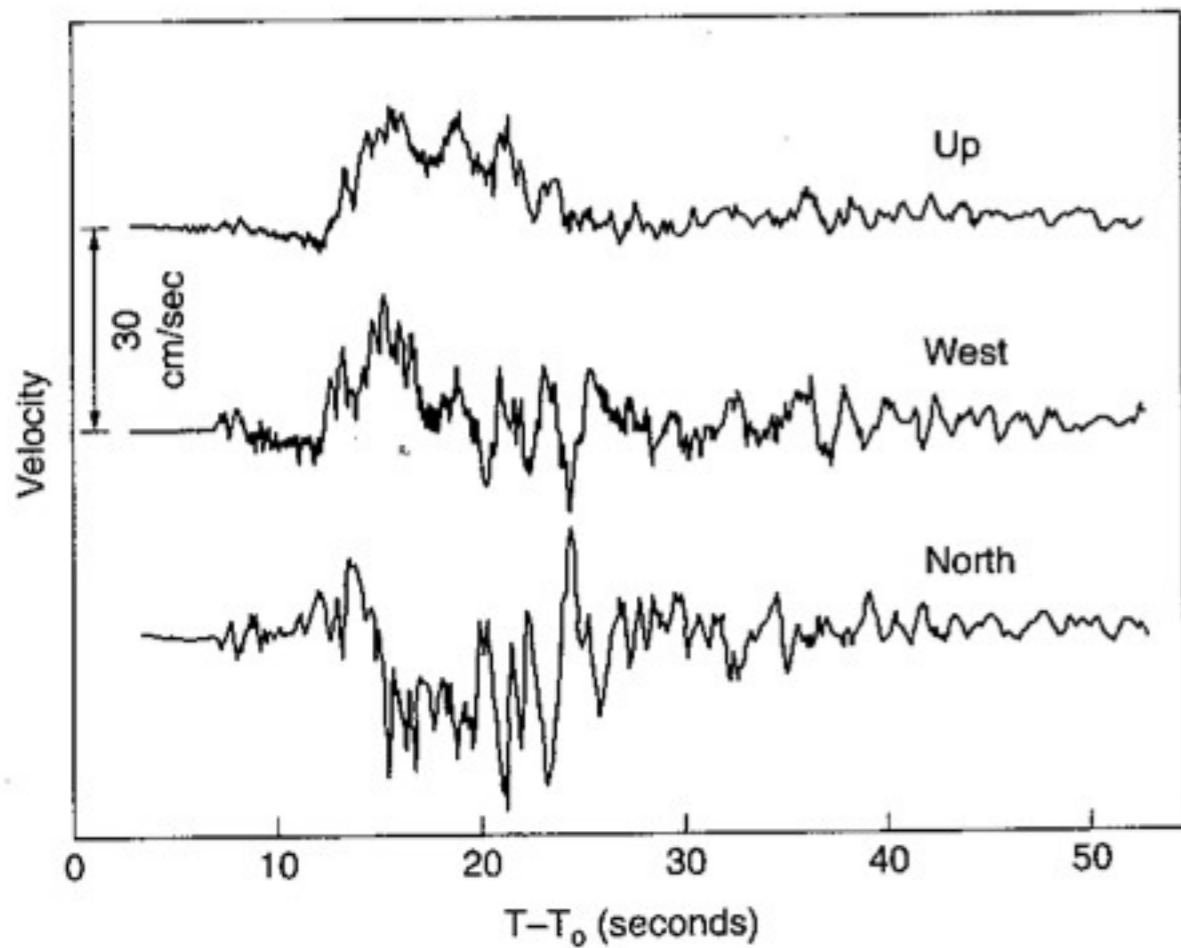


FIGURE 2 Three components of velocity at Caleta de Campos during the September 19, 1985, Michoacan earthquake (from Anderson *et al.*, 1986).

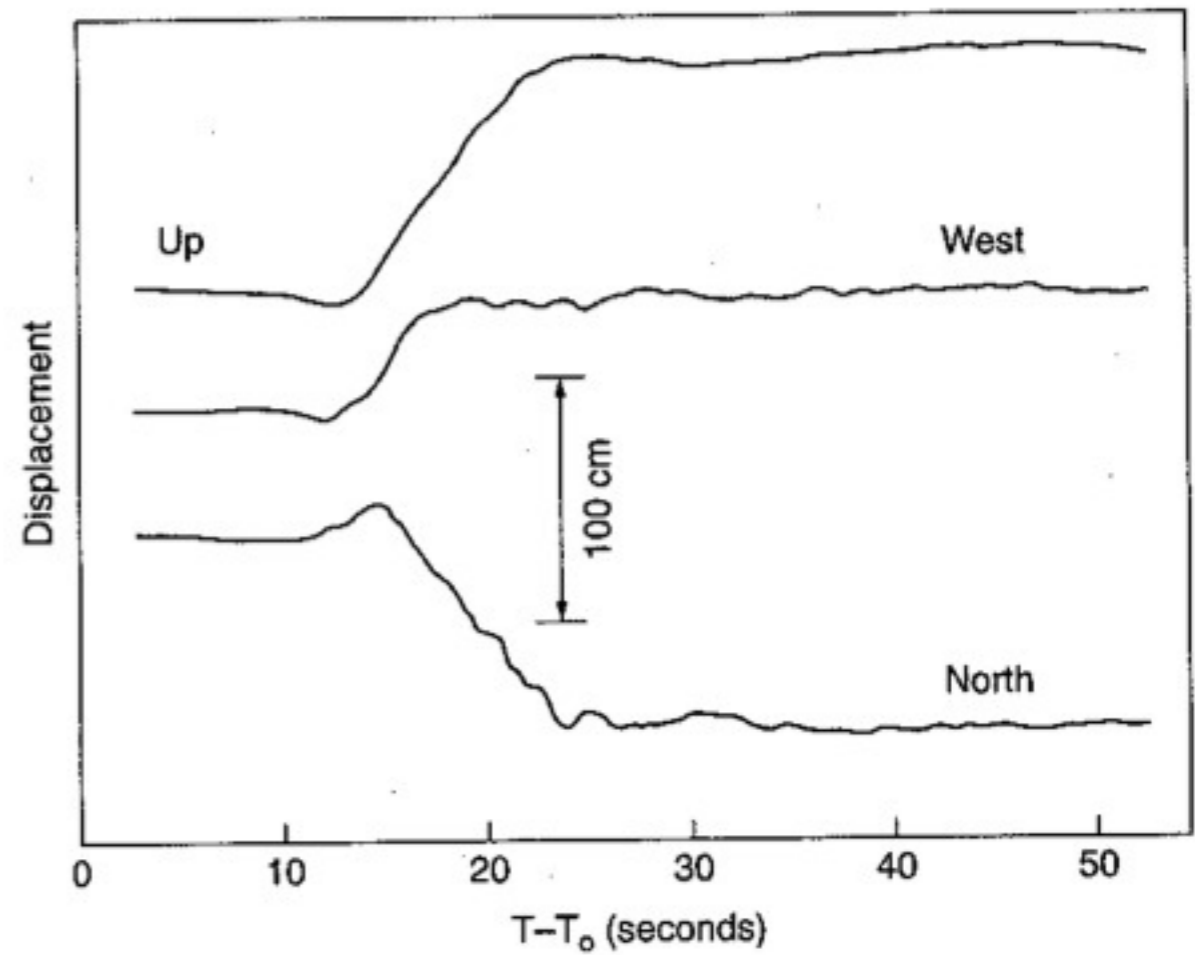


FIGURE 3 Three components of displacement at Caleta de Campos during the September 19, 1985, Michoacan earthquake (from Anderson *et al.*, 1986).

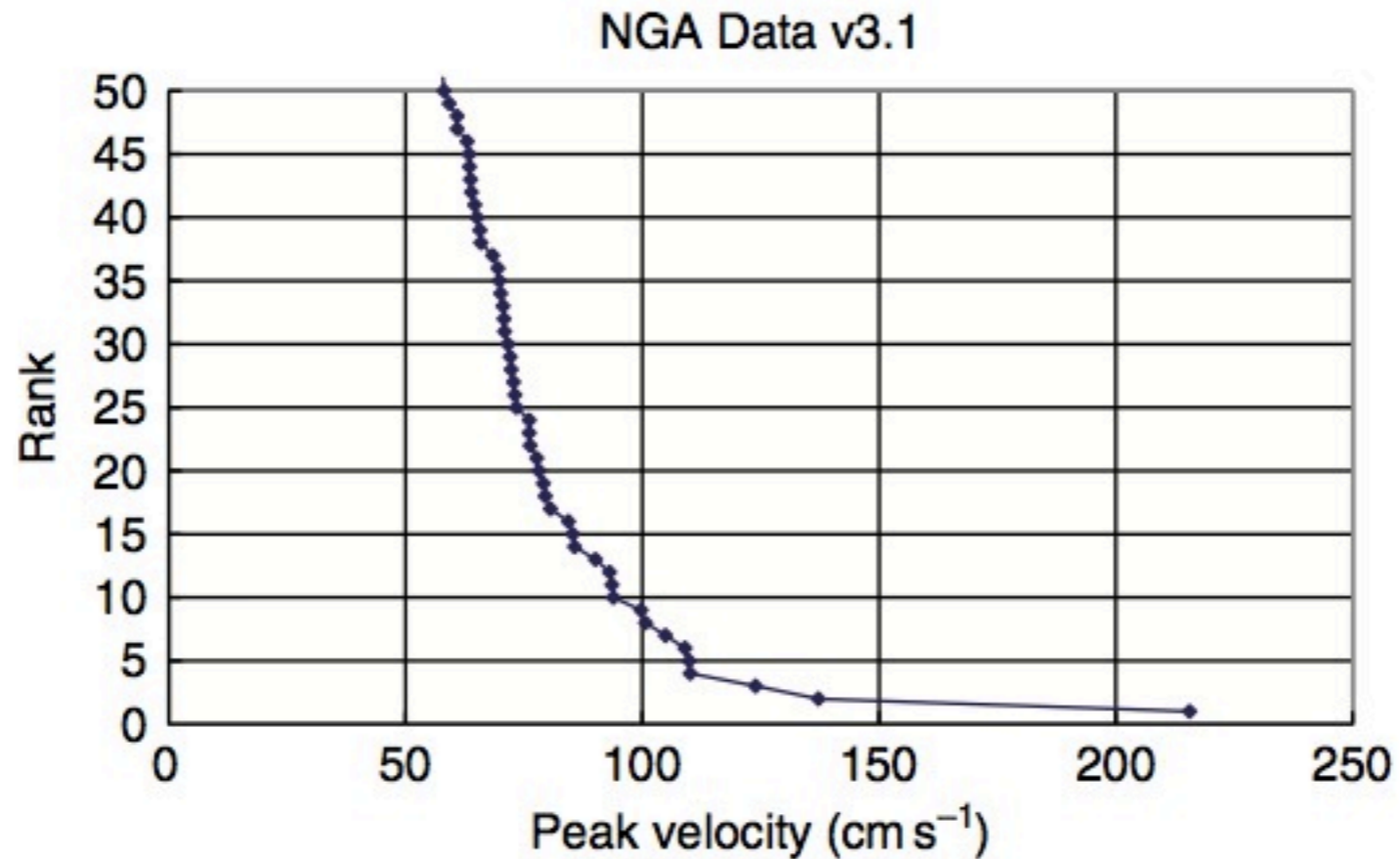


Figure 2 Extreme velocities recorded in free-field conditions. This set of observations with extreme velocity records has been drawn from the PEER NGA database. The peak velocities in this database are the geometric mean of the two horizontal components. On average, these are about 75% of the peak vector velocity. From Anderson JG, Purvance M, Brune JN, and Anooshehpour A (2005a) Developing Constraints on Extreme Ground Motions Based on Seismic and Geological Observations (extended abstract), 2nd International Workshop, Strong Ground Motion Prediction and Earthquake Tectonics in Urban Areas, 25–27 October, 2005, Earthquake Research Institute, University of Tokyo, Japan, 63–66.

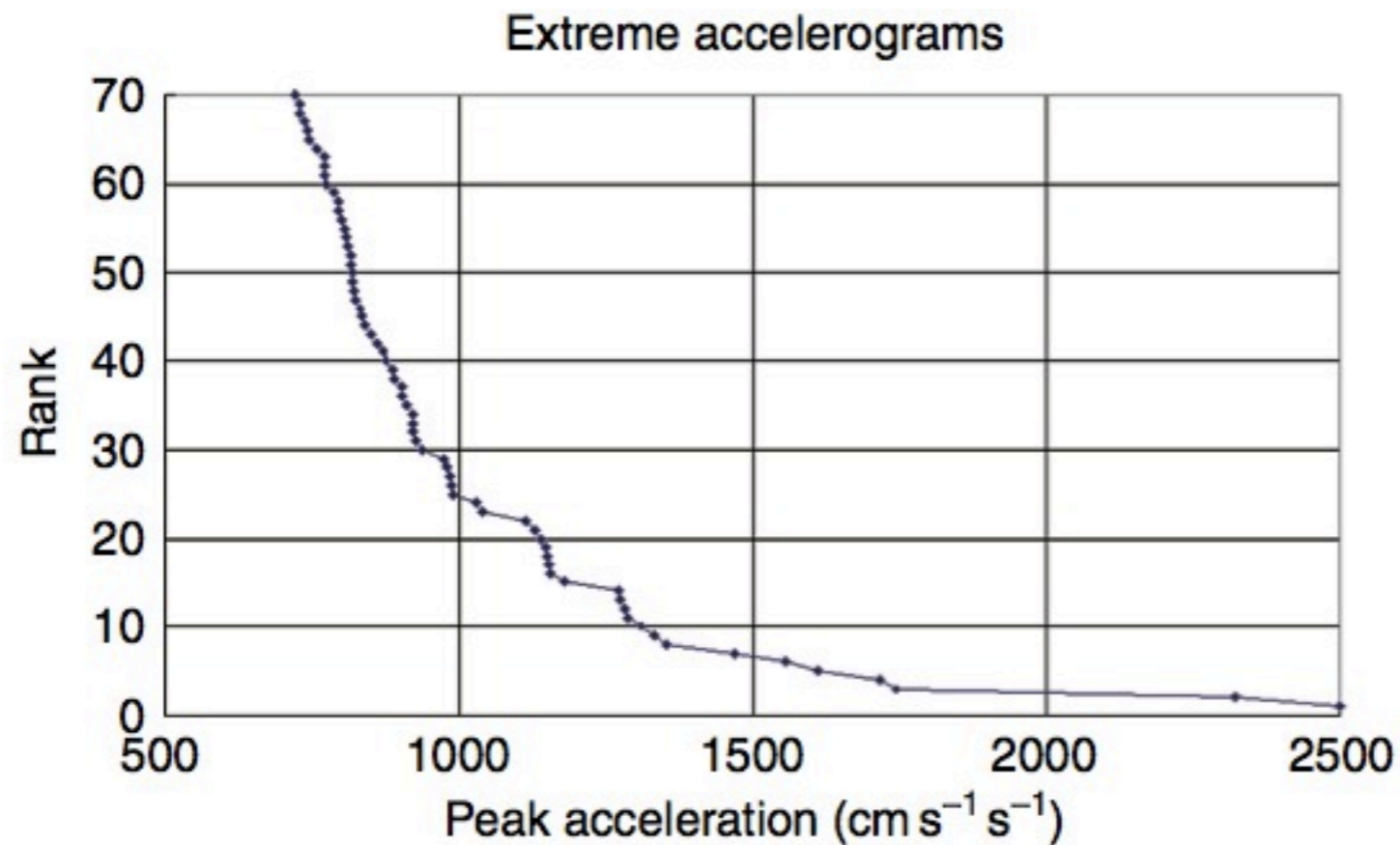


Figure 1 Extreme accelerations recorded in the ‘free-field’ or in small structures. The table with extreme records has been primarily drawn from the COSMOS Virtual Data Center. Note that these points are all peaks on individual components of strong-motion records. The peak vector acceleration is not necessarily recorded, and statistically its median is 25% greater than the largest horizontal component of a randomly oriented accelerograph. Expanded from Anderson JG, Purvance M, Brune JN, and Anooshehpour A (2005a) Developing Constraints on Extreme Ground Motions Based on Seismic and Geological Observations (extended abstract), 2nd International Workshop, Strong Ground Motion Prediction and Earthquake Tectonics in Urban Areas, 25–27 October, 2005, Earthquake Research Institute, University of Tokyo, Japan, 63–66.

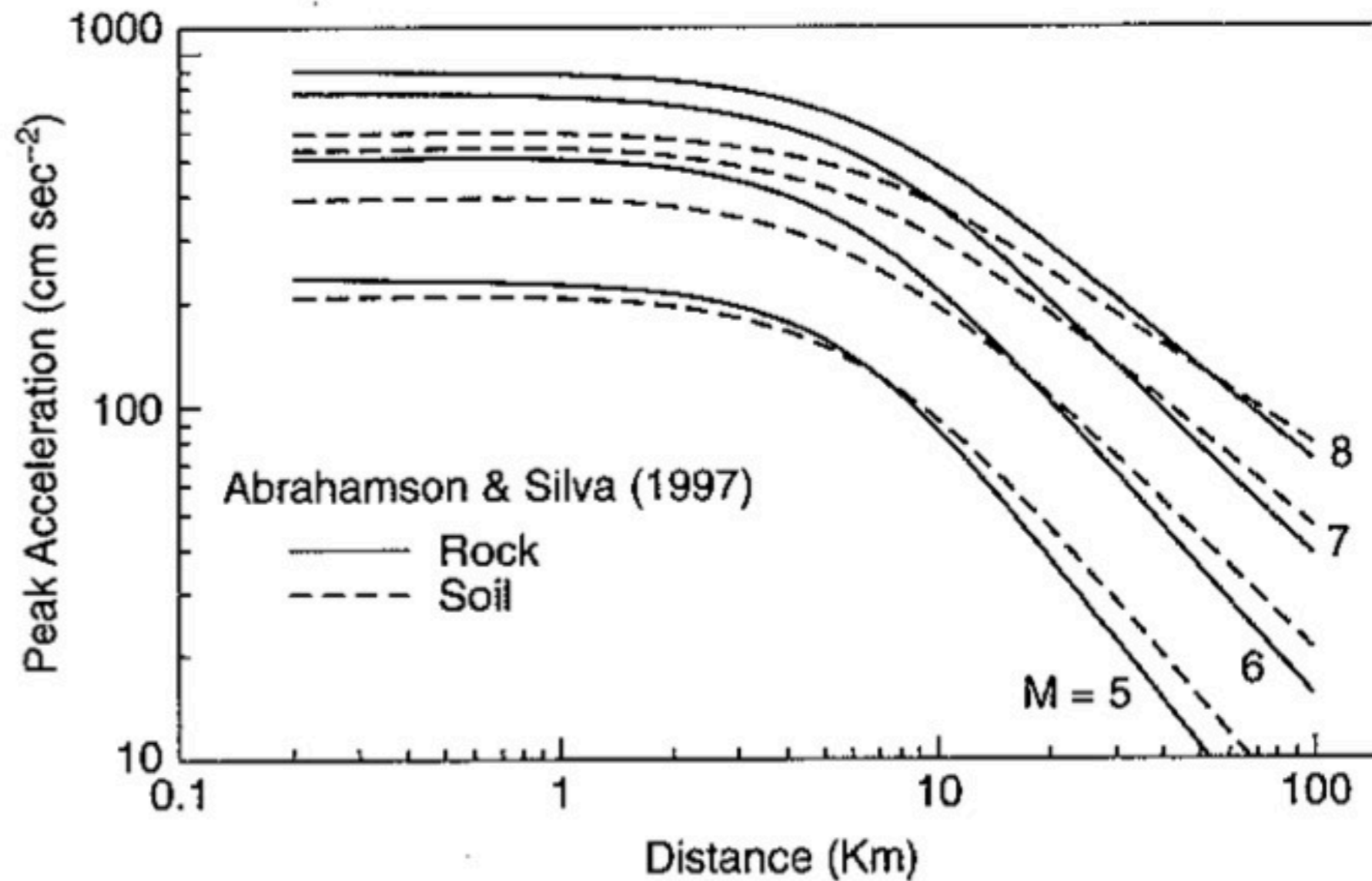
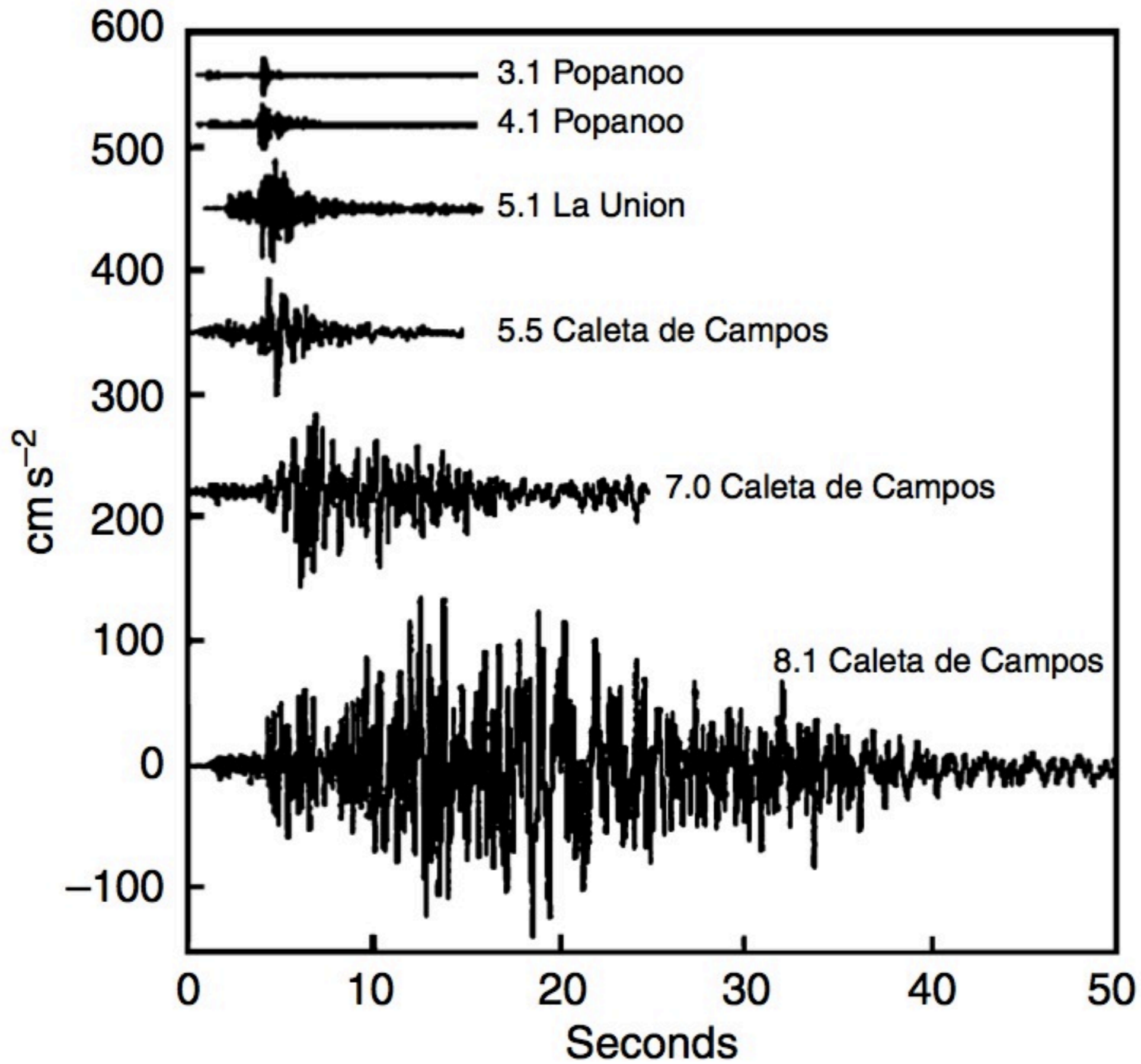
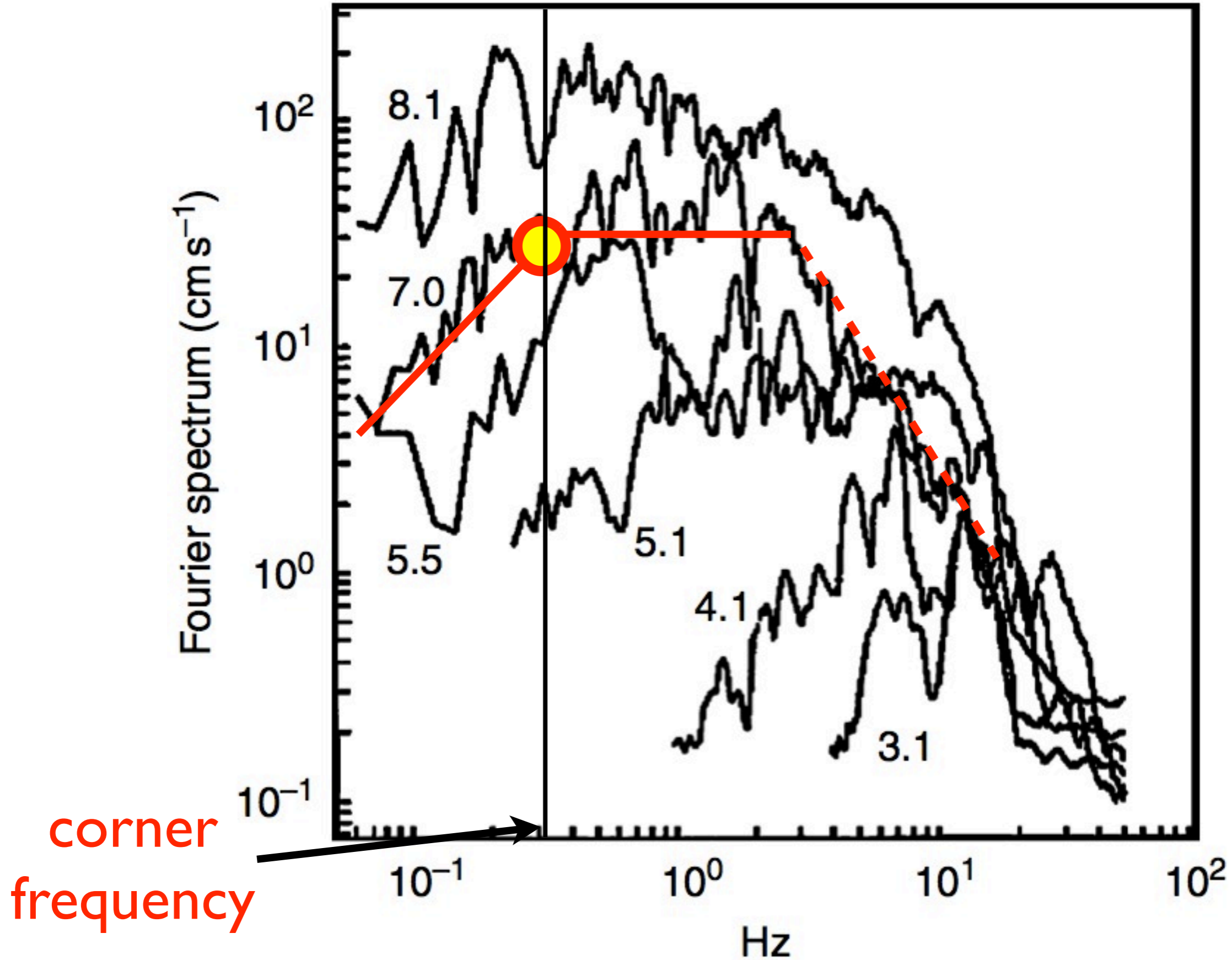
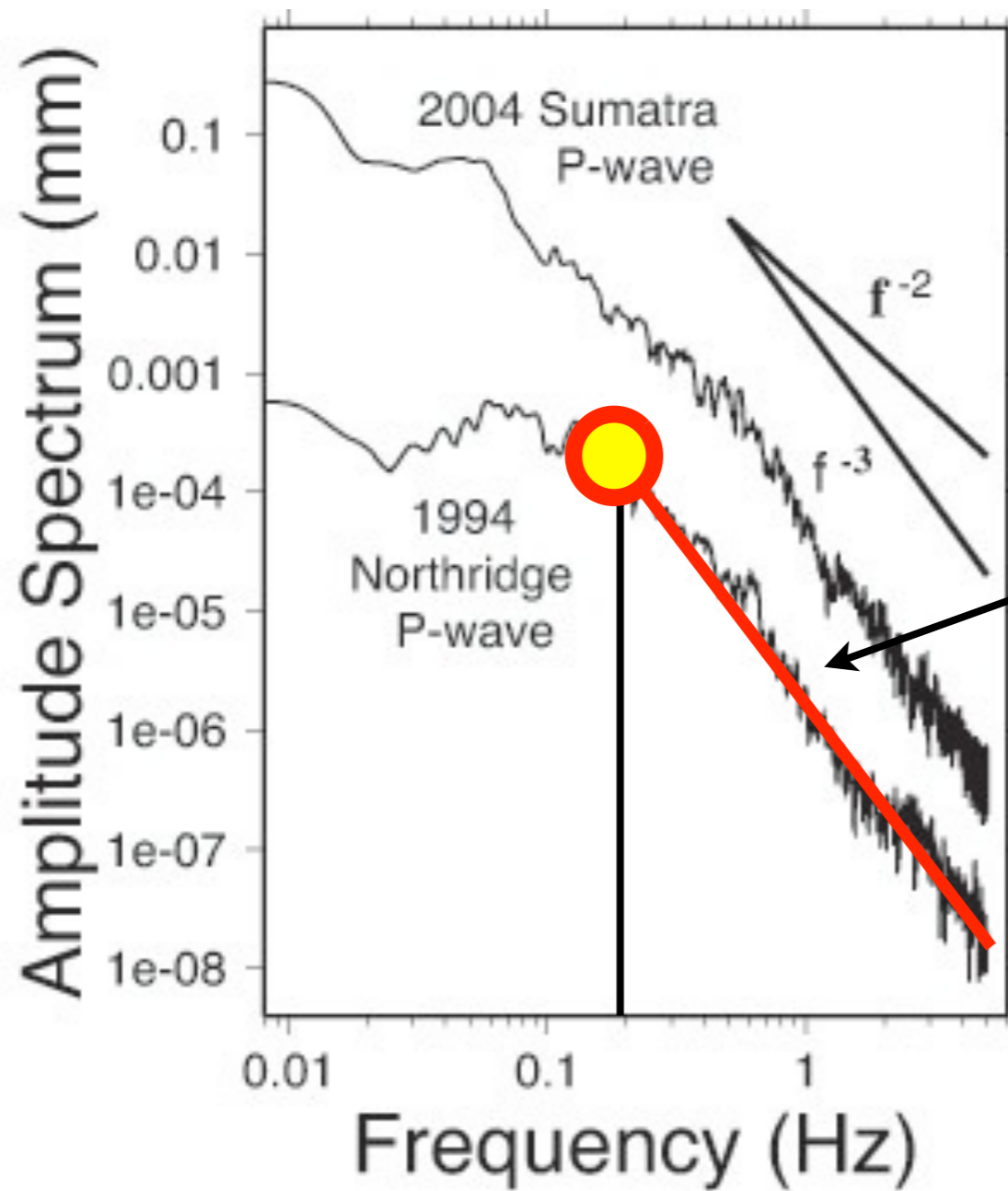


FIGURE 7 Peak acceleration as a function of magnitude and distance from the fault, as given by the ground-motion prediction equation of Abrahamson and Silva (1997).



Spectra of accelerometer records

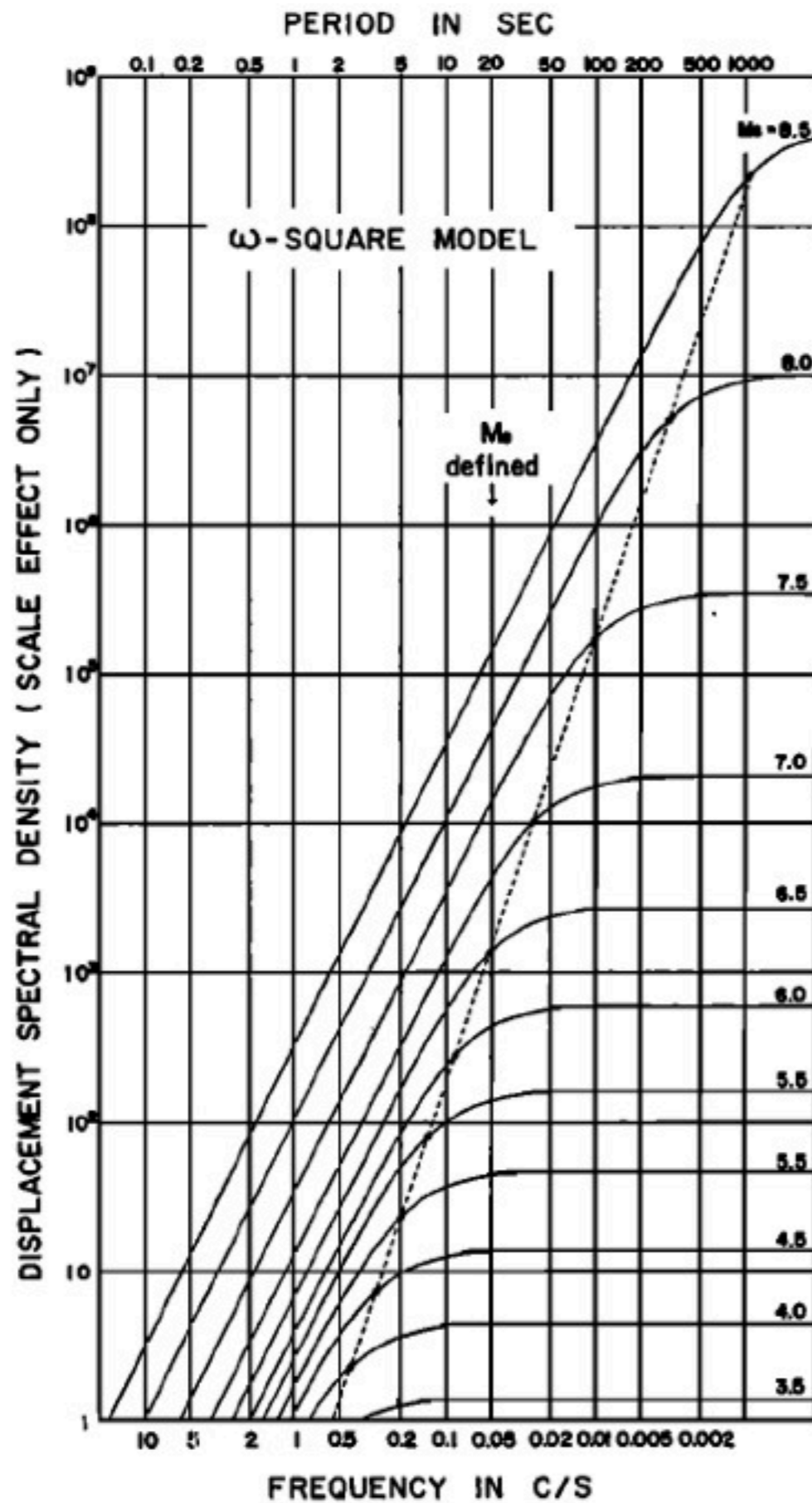




$$|\tilde{a}_{(f)}| = (2\pi f)^2 |\tilde{u}_{(f)}|$$

so the flat part of the acceleration spectrum has a slope of -2 on this displacement spectrum plot

Displacement spectrum



example: M 6
 $F_c = \underline{\hspace{2cm}}$ Hz

$$r = \frac{2.34\beta}{2\pi f_c}$$

$$\Delta\sigma = \frac{7}{16} \frac{M_0}{r^3}$$

Fig. 3. Dependence of amplitude spectral density of earthquake magnitude M , for the ω -square model.

Aki, 1967

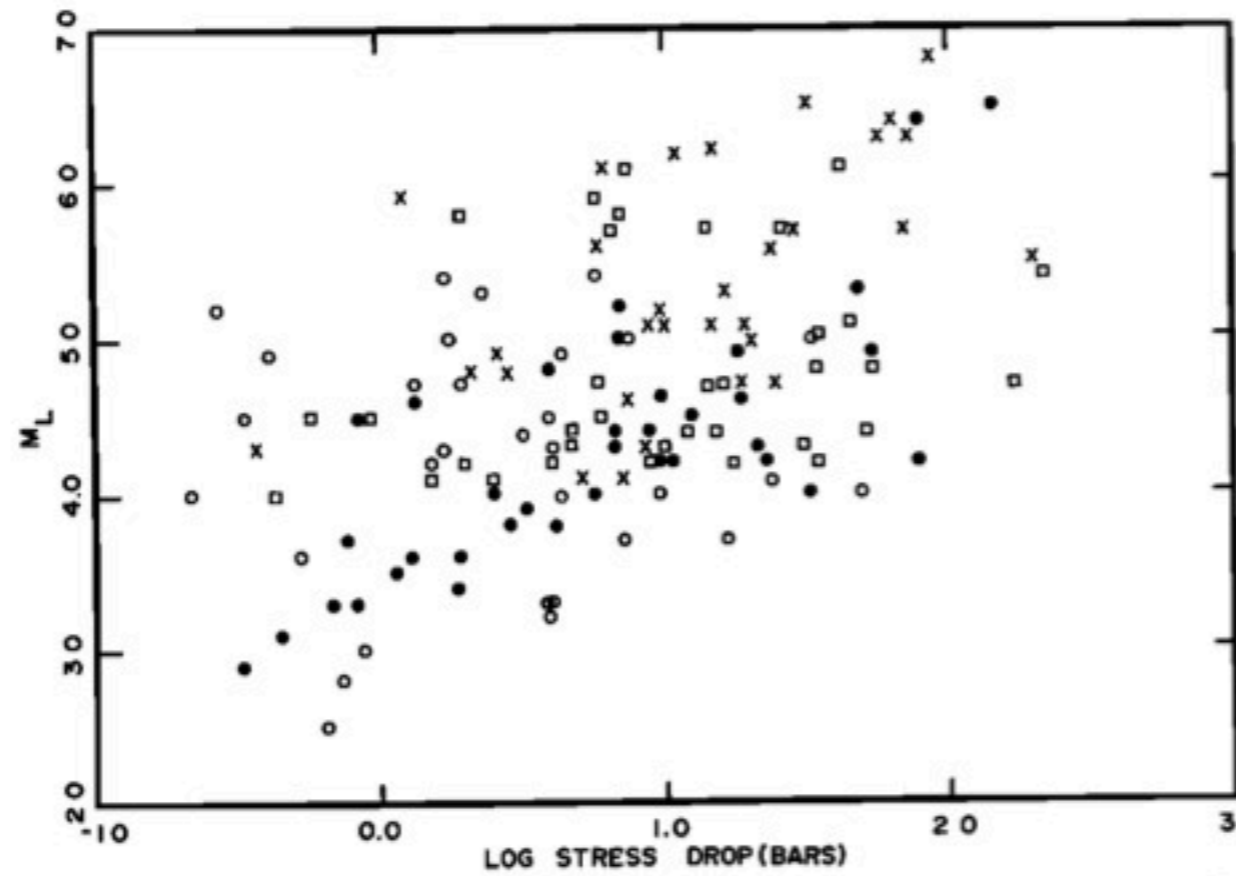
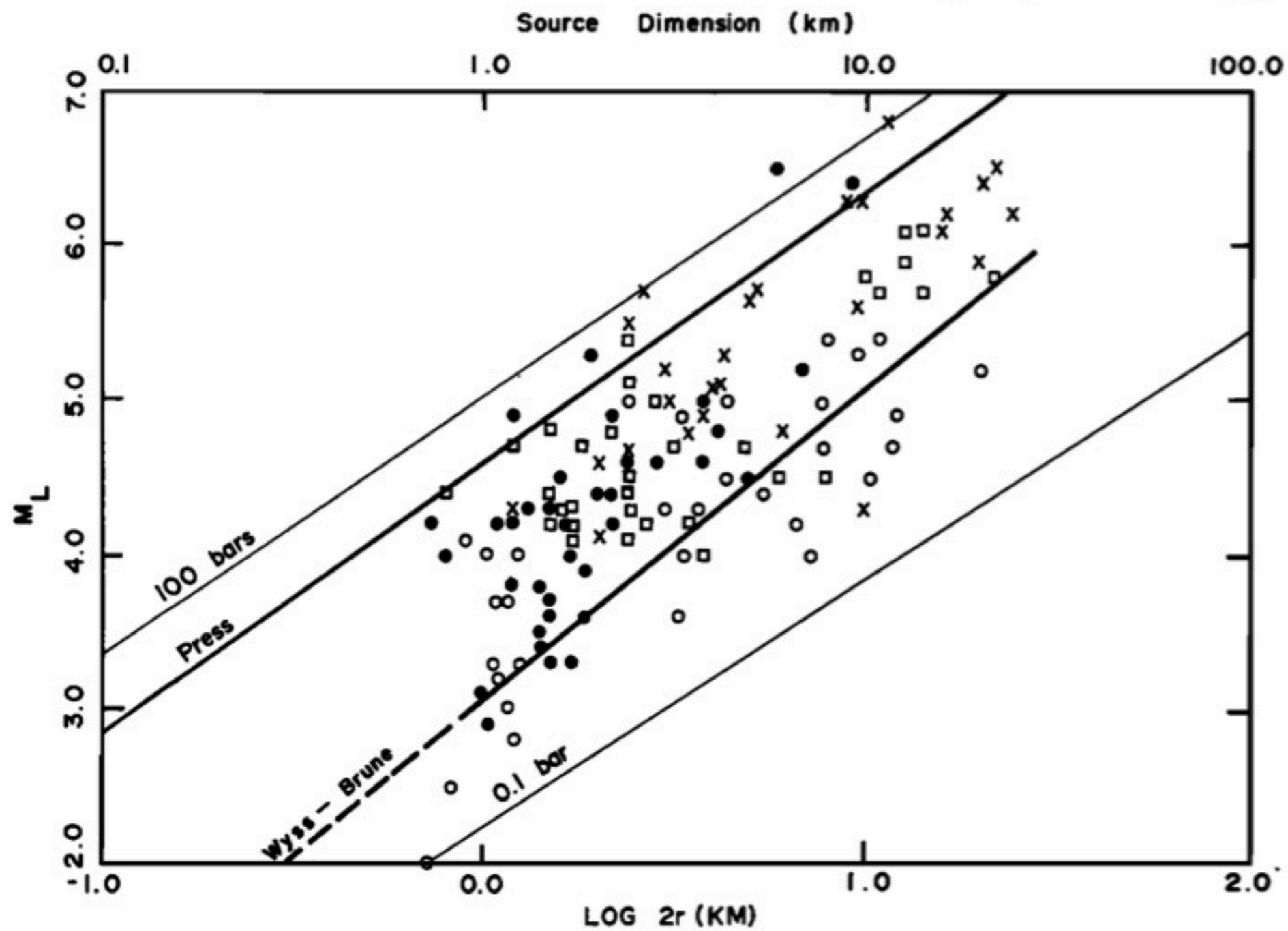
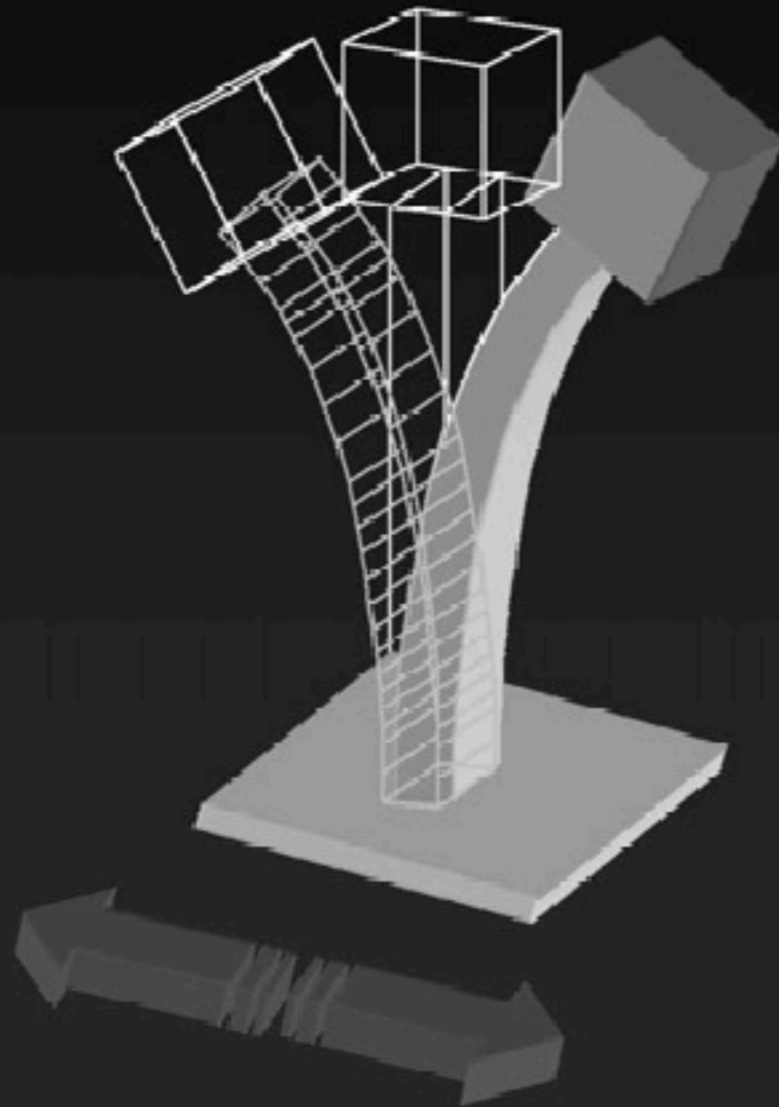


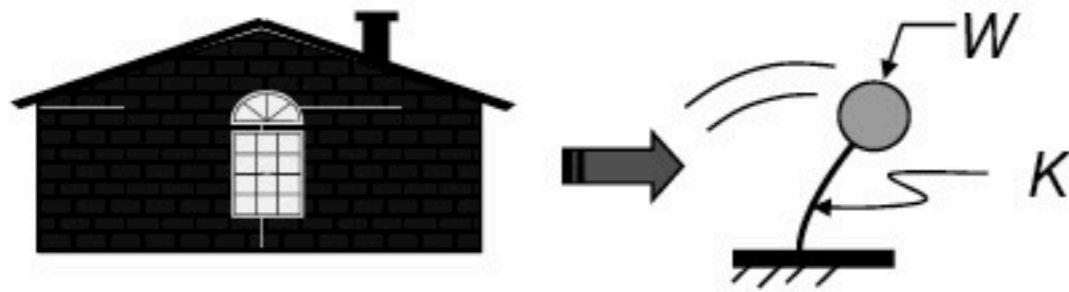
Fig. 9. Local magnitude versus log stress drop. Open circles represent earthquakes in offshore province and San Andreas fault; solid circles, in Transverse Ranges; squares, in Kern county; crosses, in other southern California locations.



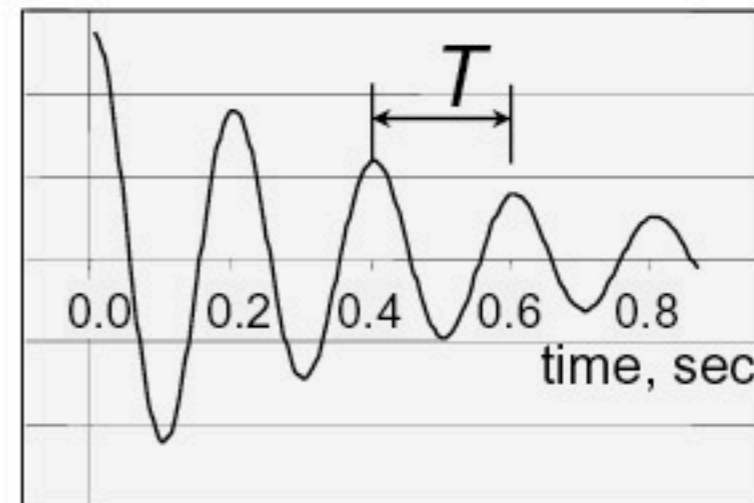
Now we impose the ground motions from an earthquake to the base of our elastic system



- Single-degree-of-freedom oscillators

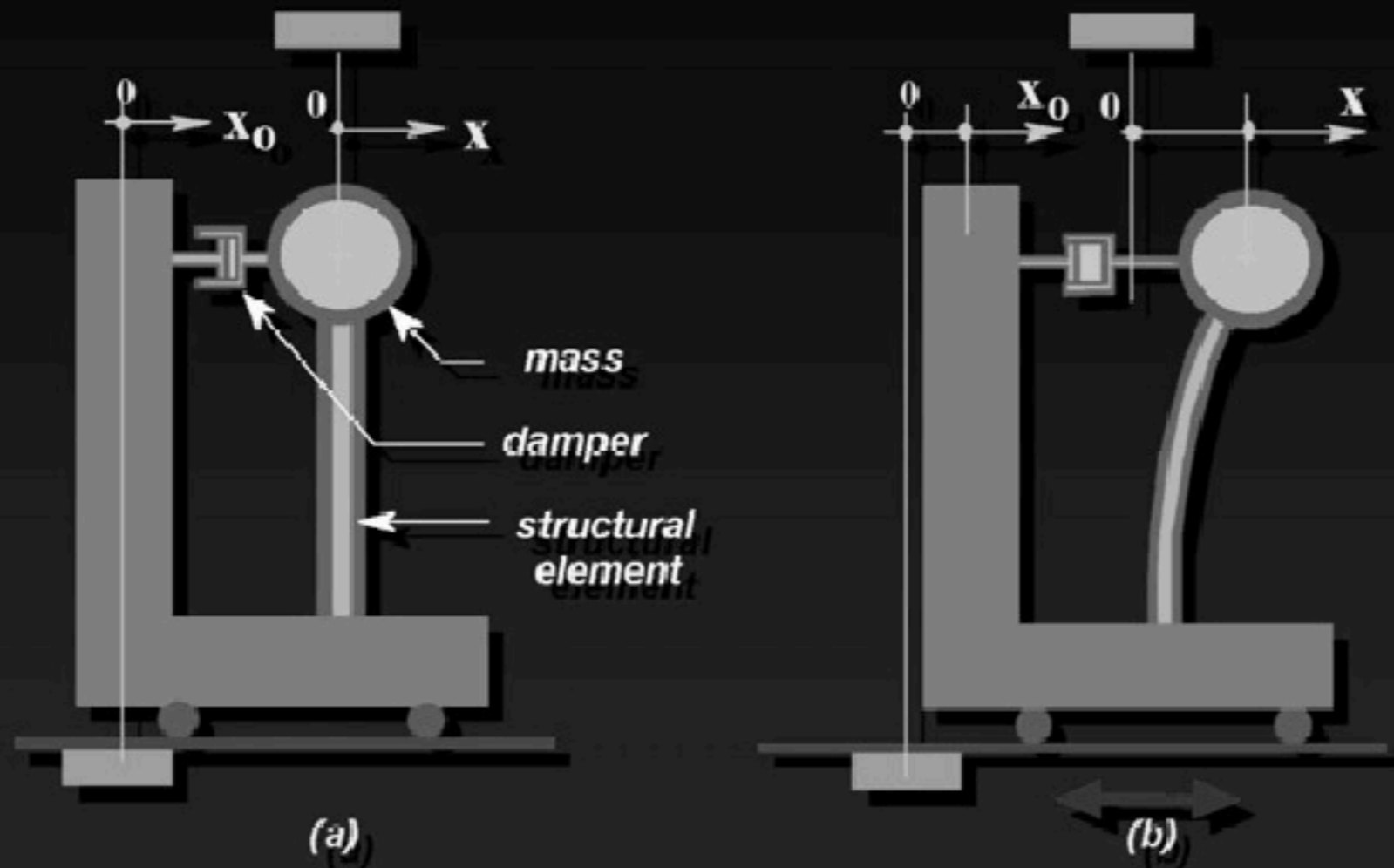


Vibration Period



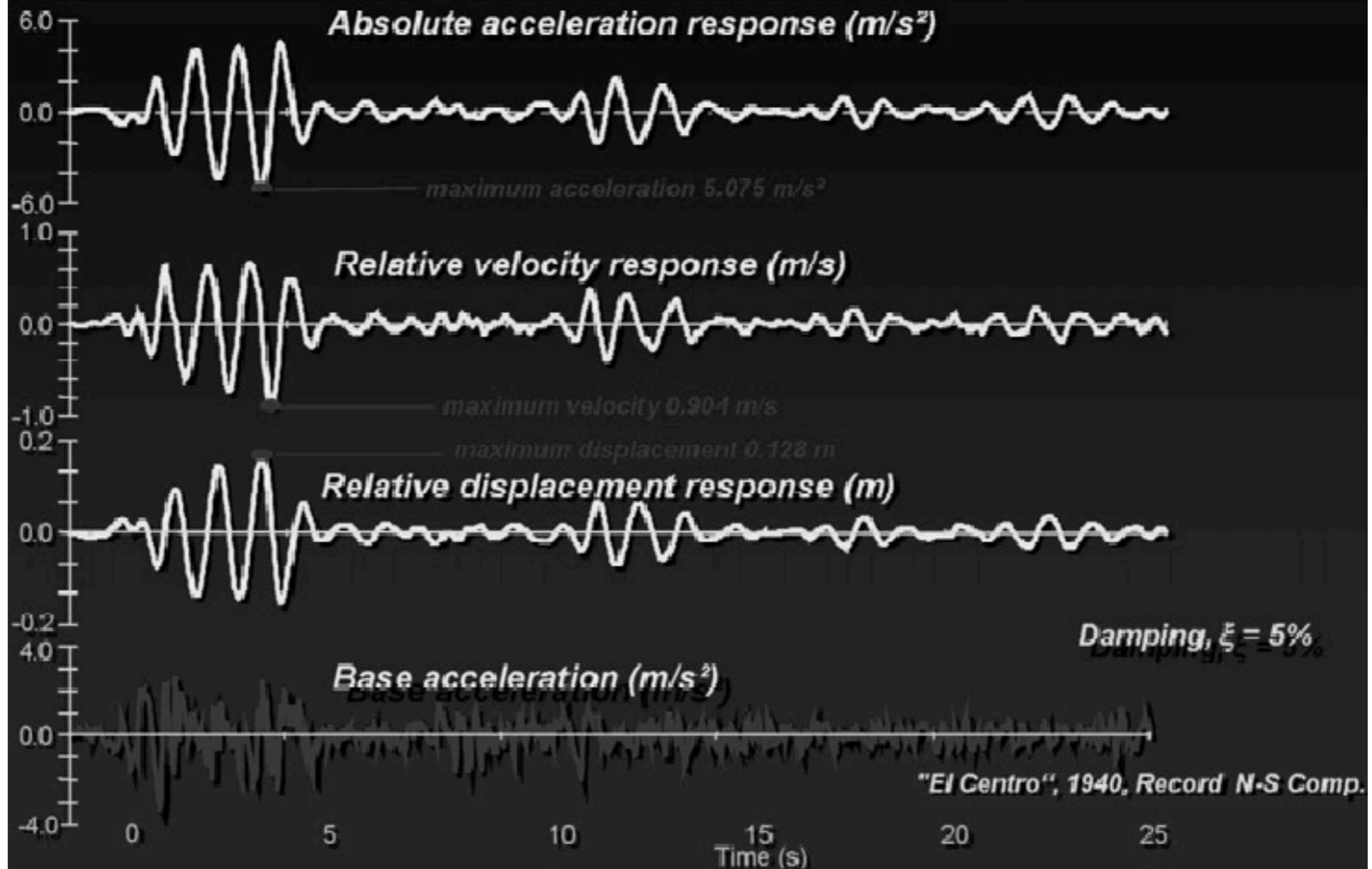
$$T = \sqrt{\frac{m}{k}}$$

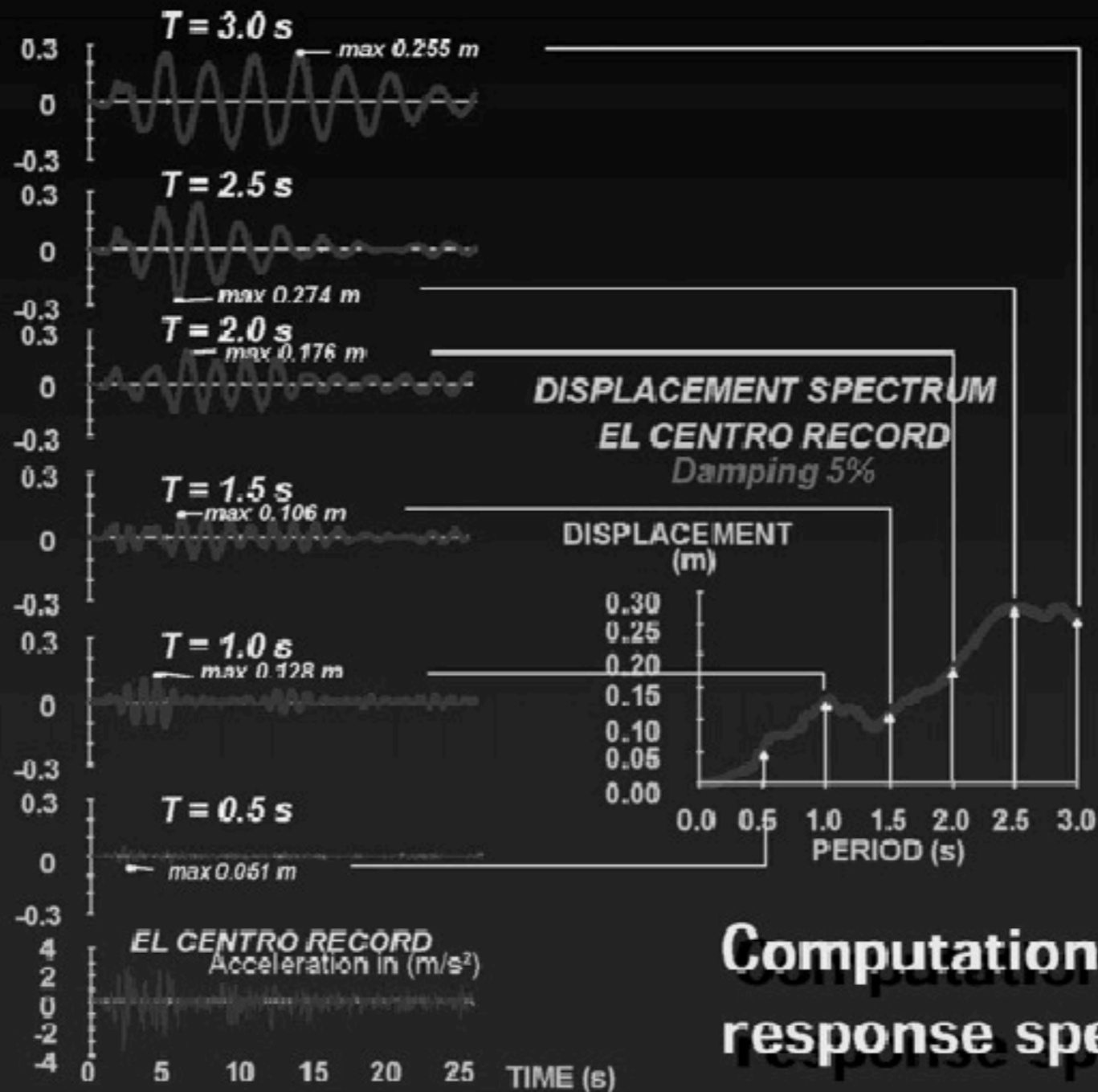
Base excitation



$$m\ddot{x} + c(\dot{x} - \dot{x}_0) + k(x - x_0) = 0$$

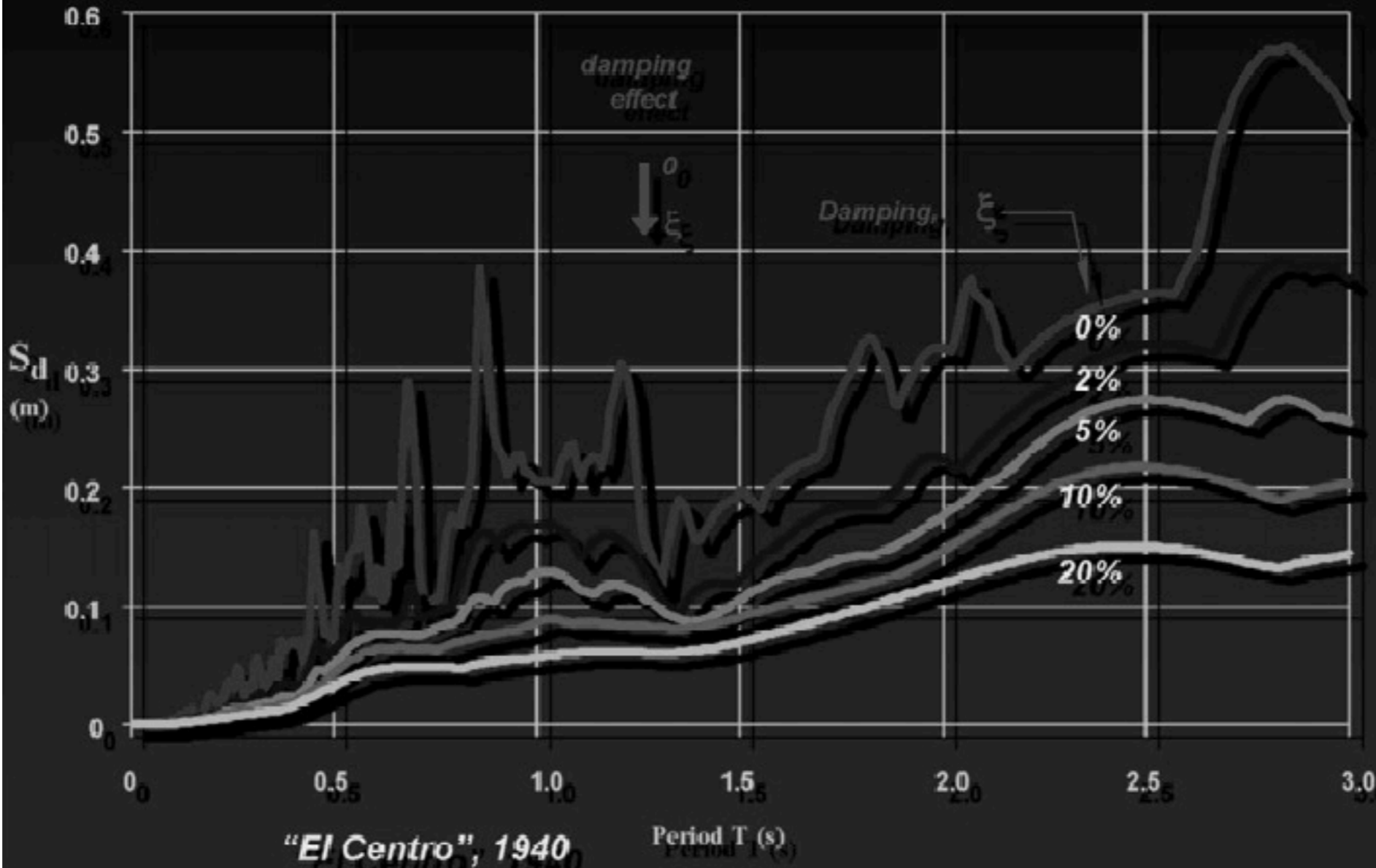
Response of a 1 second period system



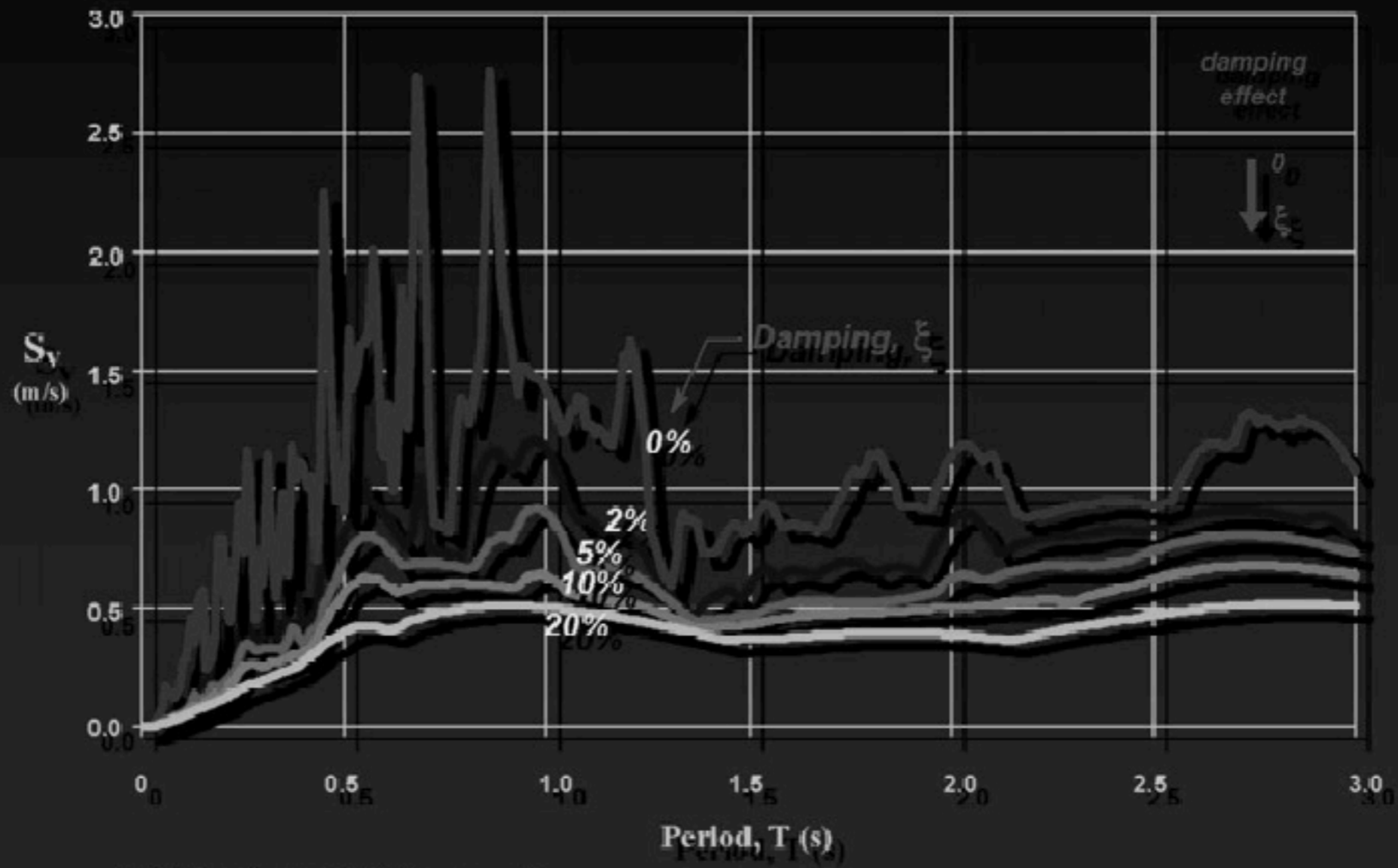


Computation of the response spectrum

Displacement spectra

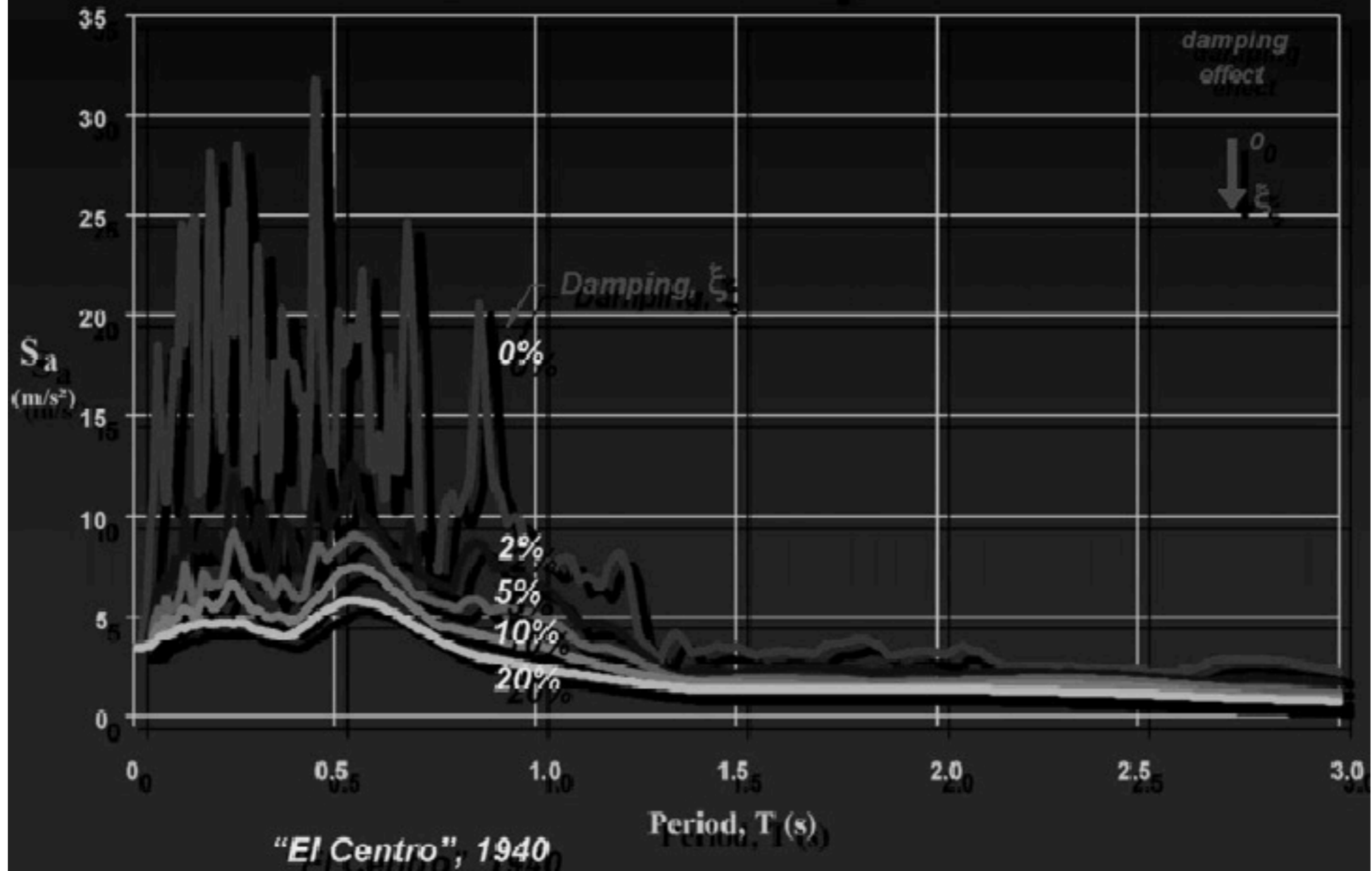


Velocity Spectra

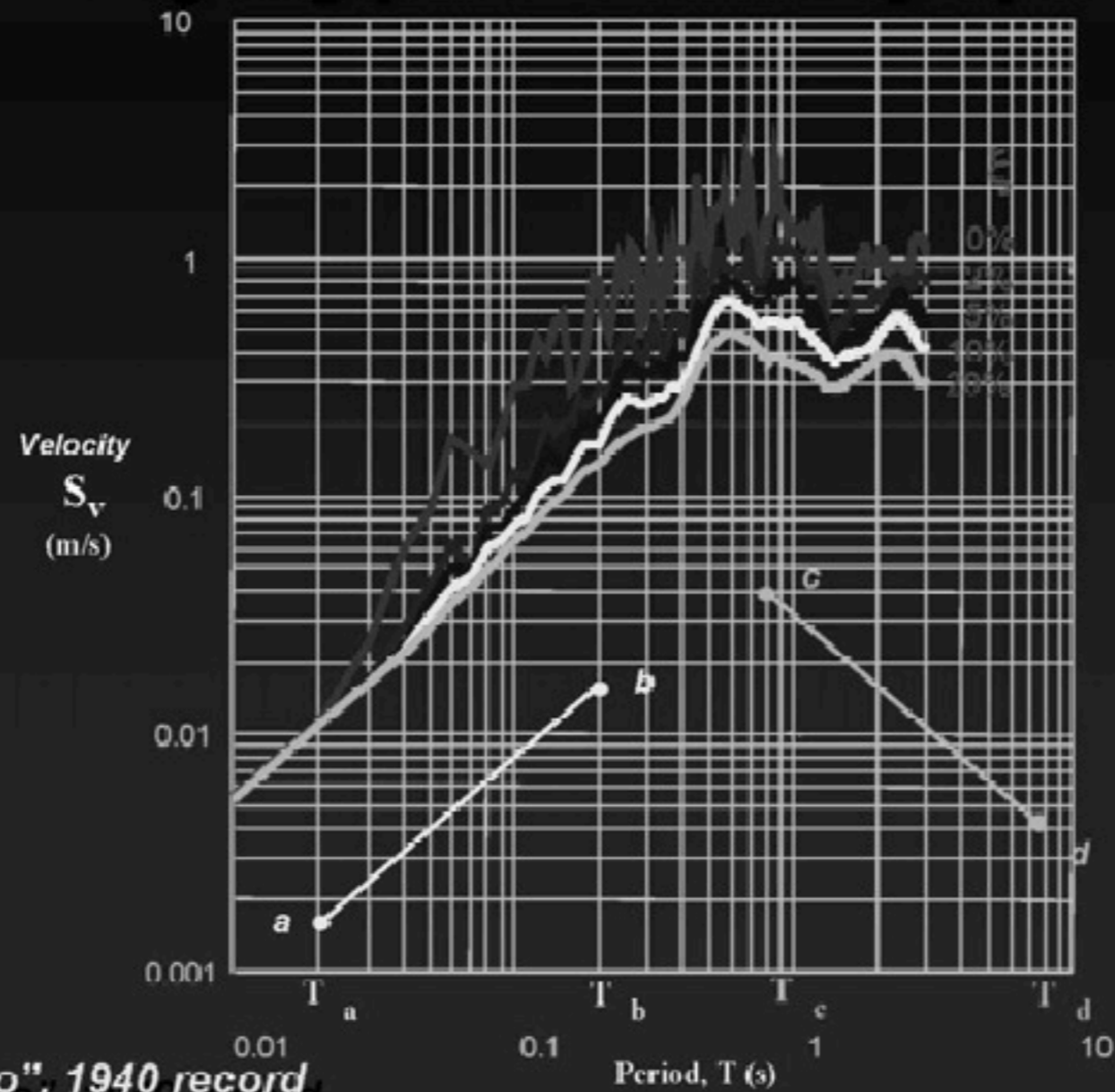


"El Centro", 1940 record

Acceleration spectra



Log-log plot of velocity spectra



"El Centro", 1940 record

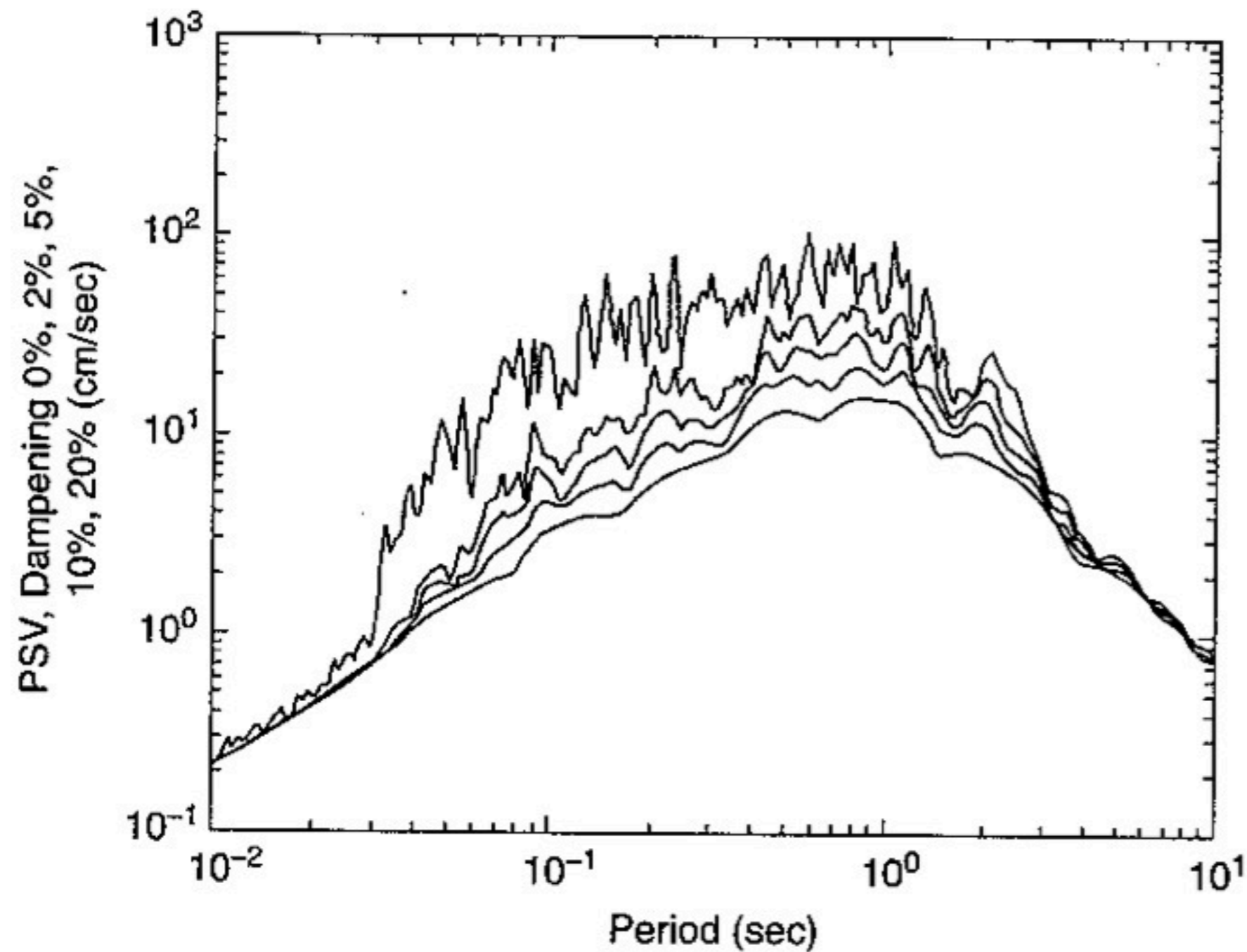


FIGURE 6 Pseudo-relative velocity response spectrum determined from the north-south component of acceleration recorded at Caleta de Campos during the September 19, 1985, Michoacan earthquake.

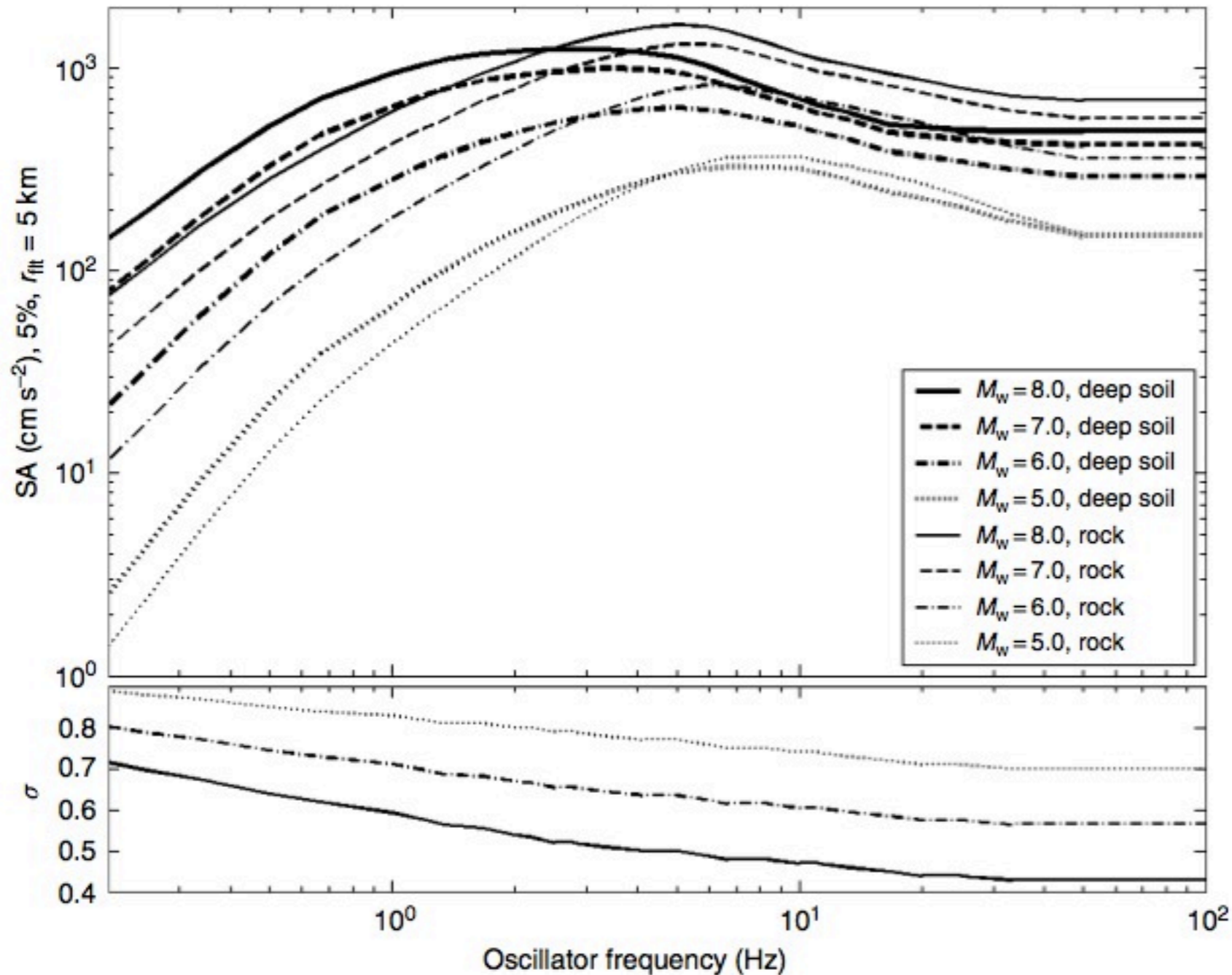


Figure 30 Ground motion predictions of Abrahamson and Silva (1997) for acceleration response spectra (SA) with 5% damping. The acceleration response at the oscillator frequency f_0 gives the peak response of a damped single-degree-of-freedom oscillator to the accelerogram. Because of its usefulness for approximating the response of a simple structure, SA is commonly used in engineering seismology instead of the Fourier spectrum (FS). At high frequencies, SA is asymptotic to peak acceleration, in contrast to the Fourier amplitude spectrum that falls off rapidly. At low frequencies, the asymptote is controlled by the peak displacement on the integrated accelerogram. At frequencies between those controlled by the asymptotes, SA is related to $(2\pi f_0)FS(f_0)$ (Udwadia and Trifunac, 1973). At the bottom, the figure shows the standard deviation of the prediction equations, in natural logarithm units. In this model, the standard deviation is the same for both soil and rock sites, and the standard deviation for $M_W = 7$ is the same as for $M_W = 8$.

$$\begin{aligned}
\mathbf{u}(\mathbf{x}, t) = & \frac{1}{4\pi\rho} \mathbf{A}^N \frac{1}{r^4} \int_{r/\alpha}^{r/\beta} \tau M_0(t - \tau) d\tau \\
& + \frac{1}{4\pi\rho\alpha^2} \mathbf{A}^{IP} \frac{1}{r^2} M_0\left(t - \frac{r}{\alpha}\right) \\
& + \frac{1}{4\pi\rho\beta^2} \mathbf{A}^{IS} \frac{1}{r^2} M_0\left(t - \frac{r}{\beta}\right) \\
& + \frac{1}{4\pi\rho\alpha^3} \mathbf{A}^{FP} \frac{1}{r} \dot{M}_0\left(t - \frac{r}{\alpha}\right) \\
& + \frac{1}{4\pi\rho\beta^3} \mathbf{A}^{FS} \frac{1}{r} \dot{M}_0\left(t - \frac{r}{\beta}\right), \tag{8}
\end{aligned}$$

where the radiation pattern terms are

$$\begin{aligned}
\mathbf{A}^N &= 9 \sin 2\theta \cos \varphi \hat{\mathbf{r}} - 6(\cos 2\theta \cos \varphi \hat{\boldsymbol{\theta}} - \cos \theta \sin \varphi \hat{\boldsymbol{\phi}}) \\
\mathbf{A}^{IP} &= 4 \sin 2\theta \cos \varphi \hat{\mathbf{r}} - 2(\cos 2\theta \cos \varphi \hat{\boldsymbol{\theta}} - \cos \theta \sin \varphi \hat{\boldsymbol{\phi}}) \\
\mathbf{A}^{IS} &= -3 \sin 2\theta \cos \varphi \hat{\mathbf{r}} + 3(\cos 2\theta \cos \varphi \hat{\boldsymbol{\theta}} - \cos \theta \sin \varphi \hat{\boldsymbol{\phi}}) \\
\mathbf{A}^{FP} &= \sin 2\theta \cos \varphi \hat{\mathbf{r}} \\
\mathbf{A}^{FS} &= (\cos 2\theta \cos \varphi \hat{\boldsymbol{\theta}} - \cos \theta \sin \varphi \hat{\boldsymbol{\phi}}). \tag{9}
\end{aligned}$$